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DISTORTIONS AND STRESSES OF PARABOLOIDAL SURFACE STRUCTURES

Part II -- The Membrane Behavior

296 491

James W. Mar Frederic Y. M. Wan

4 January 1963

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Part II -- The Membrane Behavior

Ву

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VI THE MEMBRANE BEHAVIOR: POLAR PARAMETERS

 Δ The paraboloidal shell will, under the proper circumstances, prefer to carry the applied loads by the development of the force resultants $N_{\alpha\beta}$ rather than by the development of the transverse shear resultants Q_{α} and the bending moment resultants, $M_{\alpha\beta}$. Such behavior is commonly called membrane behavior although the term -Momentless behavior is the more connotative. The membrane or momentless behavior will be the dominant mode of behavior when either the bending moments induced by the loads are negligibly small or whenever the flexural (bending) rigidity of the shell is sufficiently small so that the shell will deform without inducing appreciable bending moments. In this chapter, various solutions of the membrane equations as written in terms of the polar parameters will be presented. The so-called membrane equations are easily obtained by omitting the $M_{\alpha\beta}$, Q_{α} ,and $\hat{\kappa}_{\alpha\beta}$ terms from the equations which have been derived in the previous sections.

6.1 THE GOVERNING EQUATIONS

The equations which govern the membrane behavior of a paraboloidal shell will be summarized in this section.

The strain-displacement relations (see equations 3.4.19, 3.4.20, 3.4.21):

$$\epsilon_{\rm r}^{\circ} = \frac{1}{2f\sqrt{1+(\gamma)^2}} \frac{\partial u_{\rm r}^{\circ}}{\partial \gamma} - \frac{W}{2f[1+(\gamma)^2]^{3/2}}, \qquad 6.1.1$$

$$\epsilon_{\theta}^{\circ} = \frac{1}{2f\Upsilon} \frac{\partial u_{\theta}^{\circ}}{\partial \theta} + \frac{u_{r}^{\circ}}{2f\Upsilon\sqrt{1+(\Upsilon)^{2}}} - \frac{W}{2f\sqrt{1+(\Upsilon)^{2}}}, \qquad 6.1.2$$

$$2 \epsilon_{r\theta}^{\circ} = \frac{1}{2f\sqrt{1+(r)^2}} \frac{\partial u_{\theta}^{\circ}}{\partial r} - \frac{u_{\theta}^{\circ}}{2f\sqrt{1+(r)^2}} + \frac{1}{2f\sqrt{3}} \frac{\partial u_{r}^{\circ}}{\partial \theta} . \qquad 6.1.3$$

The equations of equilibrium (see equations 4.4.1.14, 4.4.1.15, 4.4.1.16):

$$\gamma \frac{\partial N_r}{\partial \gamma} + \sqrt{1+(\gamma)^2} \frac{\partial N_r \theta}{\partial \theta} + N_r - N_{\theta} + 2f \gamma \sqrt{1+(\gamma)^2} p_r = 0,$$

6.1.4

$$\frac{\partial N_{r\theta}}{\partial Y} + \sqrt{1 + (Y)^2} \frac{\partial N_{\theta}}{\partial \theta} + 2N_{r\theta} + 2fY \sqrt{1 + (Y)^2} P_{\theta} = 0,$$

6.1.5

$$\frac{1}{1+(\gamma)^2} N_r + N_0 + 2f \sqrt{1+(\gamma)^2} P_n = 0.$$
 6.1.6

The stress-strain relations (see equations 5.2.7, 5.2.8, 5.2.9)

$$N_{\Gamma} = \frac{Eh}{1 - \nu^2} \left(\epsilon_{\Gamma}^o + \nu \epsilon_{\theta}^o \right) , \qquad 6.1.7$$

$$N_{r\theta} = \frac{Eh}{1-\nu^2} \left(1-\nu\right) \epsilon_{r\theta}^{o} , \qquad 6.1.8$$

$$N_{\theta} = \frac{Eh}{1-\nu^2} \left(\epsilon_{\theta}^{\circ} + \nu \epsilon_{r}^{\circ} \right). \tag{6.1.9}$$

There are nine equations for the nine unknown quantities:

$$\epsilon_{r}^{\circ}, \epsilon_{re}^{\circ}, \epsilon_{e}^{\circ}, u_{r}^{\circ}, u_{e}^{\circ}, w, N_{r}, N_{re}, N_{e}$$

6.2 THE BOUNDARY CONDITIONS

We will be concerned in the main with a paraboloidal shell which is bounded by one or two boundaries located at $\delta = \delta_1$ and $\delta = \delta_2$, i.e., the boundaries are circles of latitude. These possibilities are shown in figure 6.2.1.

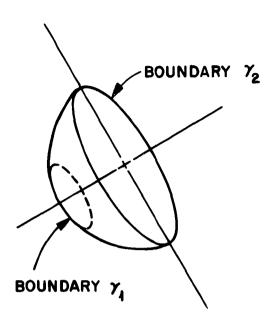


Figure 6.2.1

Note that the case where the shell is closed at the apex is not excluded here as δ_1 can well be zero.

The system of differential equations for determining the displacements is a second order system (cf. 6.1.1, 6.1.2 and 6.1.3). However, the stress resultants entering into the right hand sides of these differential equations are themselves solutions of a second order system (cf. 6.1.4, 6.1.5 and 6.1.6). Hence the displacements in the membrane theory satisfy a fourth order system of equations. Correspondingly, two independent boundary conditions must be given at each of the edges of the shell. It is clear from the equations themselves that half of these conditions must be given in terms of the displacements while the remaining conditions can be given either in terms of the displacements or the stress resultants.

It should be observed that the number of boundary conditions which can be

satisfied in the membrane theory is only half of that satisfied in the general theory. This phenomenon stems from the basic assumption in membrane theory that the shell has no bending stiffness in which case the unknowns $M_{\alpha\beta}$, Q_{α} , and $k_{\alpha\beta}$ need not be considered.

In the case of a shell with both edges restrained against tangential motion, the boundary conditions are all given in terms of the displacements. They are

$$u_{h}^{o}(x_{1}, \theta) = 0,$$
 6.2.1

$$u_r^o(x_2, \theta) = 0,$$
 6.2.2

$$u_{\theta}^{\circ}(\delta_{1}, \theta) = 0,$$
 6.2.3

$$u_0^{\circ}(X_2, \Theta) = 0.$$
 6.2.4

On the other hand, the boundary conditions for a shell with one edge free, δ_1 , and the other edge, δ_2 , restrained against tangential motion are

$$N_{n}(Y_{1}, \theta) = 0, \qquad 6.2.5$$

$$N_{re}(y_1, \theta) = 0,$$
 6.2.6

$$u_n^o(y_2, \theta) = 0,$$
 6.2.7

$$u_{\bullet}^{\circ}(I_{2},\Theta)=0.$$
 6.2.8

If the shell is closed at the apex, the usual procedure is to require that the stress resultants be finite there.

It will be shown in a subsequent section that, under the conditions enumerated in equations 6.2.1 to 6.2.8, the shell exhibits no inextensional deformations (i.e. deformations without straining the middle surface of the shell). The corresponding membrane analysis will be simplified considerably.

6.3 THE LOADS DUE TO GRAVITY

A type of loading which is of great interest is that which is caused by the dead weight of the shell, i.e., the so-called gravity loading. In this section we will express the loading intensities p_r , p_e , p_n in terms of the gravity loading. Consider the case where the shell has rotated from its face-up position (see figure 6.3.1) though an angle ψ .

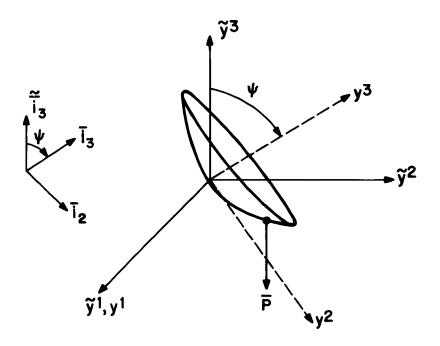


Figure 6.3.1

The $\tilde{\gamma}^n$ axes are fixed in space such that the gravity axis is in the negative $\tilde{\gamma}^3$ direction. Without loss of generality, we assume the shell to have rotated about its own γ^1 axis which coincides with the $\tilde{\gamma}^1$ axis. Let the load intensity vector due to the force of gravity be denoted by $\bar{\rho}$. Then

$$\bar{\rho} = -\rho_0 h^{\frac{\omega}{i}}_3$$

6.3.1

where ρ_0 is the weight density of the material in the shell (units of pounds per unit volume), h is the thickness, and $\frac{2}{i}$ is the unit vector in the $\frac{2}{3}$ direction. With respect to the coordinates y^n which are fixed in the shell, we have the simple relationship

$$\frac{2}{i_3} = -i_2 \sin \psi + i_3 \cos \psi \qquad 6.3.2$$

where \overline{i}_2 and \overline{i}_3 are the unit vectors associated with y^2 and y^3 . Thus

$$\bar{p} = \rho_0 h \sin \psi \bar{i}_2 - \rho_0 h \cos \psi \bar{i}_3$$
. 6.3.3

In the shell coordinates (see equation 4.4.1.19) the load intensity vector is expressed in terms of the coordinates on the middle surface of the shell.

$$\bar{P} = Pr \frac{\bar{a}_1}{\sqrt{a_{11}}} + Pe \frac{\bar{a}_2}{\sqrt{a_{22}}} + p_n \bar{n}$$
 6.3.4

where

$$P_{r} = \bar{P} \cdot \frac{\bar{a}_{1}}{\sqrt{a_{11}}} = \frac{\rho_{o}^{h}}{\sqrt{1+(\bar{Y})^{2}}} \quad [\sin \theta \sin \psi - \bar{Y}\cos \psi],$$
6.3.5

$$P_{\theta} = \bar{\rho} \cdot \frac{\bar{\alpha}_2}{\sqrt{\alpha_{22}}} = \rho_0 h \cos \theta \sin \psi, \qquad 6.3.6$$

$$p_n = \bar{\rho} \cdot \bar{n} = [-1 \sin \theta \sin \psi - \cos \psi] \frac{\rho_0 h}{\sqrt{1 + (1)^2}}$$
 6.3.7

In obtaining equations 6.3.5, 6.3.6, and 6.3.7, we have made use of the relations connecting the base vectors \overline{a}_i , \overline{a}_2 , \overline{n} , and \overline{i}_2 , \overline{i}_3 , (see equations 2.1.8, 2.1.9, 2.1.14a).

6.4 THE AXI-SYMMETRIC MEMBRANE BEHAVIOR: GRAVITY LOADS

If the structural configuration of the shell is axi-symmetric and if the loading is also axi-symmetric as in the case of the loading due to gravity, then the membrane equations become further simplified.

The axi-symmetric problem is characterized by

$$\frac{\delta}{\delta \theta} = 0, \qquad 6.4.1$$

$$u_0^0 = 0,$$
 6.4.2

$$P_0 = 0$$
 6.4.3

and the remaining partial derivative becomes an ordinary derivative, $\frac{d}{dy}$. For the validity of this statement and the uniqueness of the solution

sought, readers are referred to section 6.6.

Under these restrictions the strain-displacement relations become (see equations 6.1.1, 6.1.2, 6.1.3):

$$\epsilon_r^0 = \frac{1}{2f\sqrt{1+(\gamma)^2}} \frac{du_r^0}{d\xi} - \frac{w}{2f[1+(\gamma)^2]^3/2},$$
 6.4.4

$$\epsilon_{\theta}^{\circ} = \frac{u_{r}^{\circ}}{2f \gamma \sqrt{1+(\gamma)^{2}}} - \frac{w}{2f \sqrt{1+(\gamma)^{2}}}$$
 6.4.5

The equations of equilibrium also become simplified (see equations 6.1.4, 6.1.5, 6.1.6):

$$\sqrt[8]{\frac{dN_r}{dx}} + N_r - N_\theta + 2f \sqrt[8]{1 + (x)^2} \quad P_r = 0,$$
6.4.6

$$\frac{1}{1+(\gamma)^2} N_r + N_\theta + 2f\sqrt{1+(\gamma)^2} P_n = 0.$$
 6.4.7

The pertinent stress-strain relations (see 6.1.7, 6.1.8, 6.1.9)

are the following:

$$N_r = \frac{Eh}{1-\nu^2} \left(\epsilon_r^0 + \nu \epsilon_\theta^0 \right), \qquad 6.4.8$$

$$N_{\theta} = \frac{Eh}{1-\nu^2} \left(\epsilon_{\theta}^0 + \nu \ \epsilon_{r}^0 \right).$$
 6.4.9

The loading which is of interest is caused by the dead weight of the shell. From section 6.3 we have

$$p_r = -\frac{\rho_0 h \Upsilon}{\sqrt{1 + (\gamma)^2}}$$
, 6.4.10

$$p_{A} = 0$$
, 6.4.11

$$p_n = \frac{-\rho_0 h}{\sqrt{1 + (r)^2}}$$
 6.11.12

With the introduction of the loading terms (equations 6.4.10, 6.4.12) the two equilibrium equations can be combined into a single ordinary differential equation:

$$\frac{dN_{\Gamma}}{dY} + N_{\Gamma} \left[\frac{1}{Y} + \frac{1}{Y \left[1 + (Y)^{2} \right]} \right] = 2fh \rho_{0} \left(Y + \frac{1}{Y} \right)$$
 6.4.13

The formal solution of the differential equation (6.4.13) can be expressed as

$$N_{r} = \frac{2f\rho_{0}h}{3} \frac{\sqrt{1+(\gamma)^{2}}}{(\gamma)^{2}} \left\{ \left[1+(\gamma)^{2} \right]^{3/2} + C_{1} \right\}$$
 6.4.14

where C_{η} is a constant to be determined by the boundary conditions.

Note that the differential equation (6.4.13) possesses a regular singularity at the apex, Y=0. Thus the solution, as it is presented in equation 6.4.14, is valid only for Y>0, i.e., for all $Y\geq 8$ where 8>0 can be arbitrarily small. The case where the shell is closed at the apex will be discussed later. However, we may avoid the difficulties associated with the singularity by constructing the paraboloidal shell with a hole at the apex.

The force resultant No is obtained by substituting

equation 6.4.14 into the equilibrium equation (6.4.7):

$$N_{\theta}(x) = 2f_{\rho_0}h \left\{ 1 - \frac{1}{3(x)^2 \sqrt{1+(x)^2}} \left[(1+(x)^2)^{3/2} + C_1 \right] \right\}.$$
 6.4.15

We will require the displacements $\mathbf{u}_{\mathbf{r}}^{\mathbf{0}}$, $\mathbf{u}_{\mathbf{\theta}}^{\mathbf{0}}$, and \mathbf{w} not only because the deformations are of primary interest but also because all the boundary conditions may be prescribed in terms of them. By solving equations 6.4.8 and 6.4.9 for $\boldsymbol{\epsilon}_{\mathbf{r}}^{\mathbf{0}}$ and $\boldsymbol{\epsilon}_{\mathbf{\theta}}^{\mathbf{0}}$, and then introducing equations 6.4.14 and 6.4.15, the strains can be determined:

$$\epsilon_{r}^{0} = \frac{2f\rho_{0}}{3E} \left\{ \frac{1+\nu}{(x)^{2}} + 2(1-\nu) + (x)^{2} + c_{1} \left[\frac{(1+\nu)}{(x)^{2}\sqrt{1+(x)^{2}}} + \frac{1}{\sqrt{1+(x)^{2}}} \right] \right\}, \quad 6.14.16$$

$$\epsilon_{\theta}^{\circ} = \frac{2f\rho_{\circ}}{3E} \left\{ 2(1-\nu) - \frac{1+\nu}{(\gamma)^{2}} - \nu(\gamma)^{2} - C_{1} \left[\frac{(1+\nu)}{(\gamma)^{2}} + \frac{1}{\sqrt{1+(\gamma)^{2}}} \right] \right\}$$
 6.4.17

The strain-displacement relations (6.4.6) can be re-

arranged to read

$$w = \frac{u_r^0}{r} - 2f \sqrt{1 + (r)^2} \in \theta$$
 6.4.18

and substituted into the other strain-displacement relation (6.4.4) to yield the differential equation for u_r^{\bullet} .

Ş

$$\frac{du_{r}^{0}}{dx} - \frac{u_{r}^{0}}{x[1+(x)^{2}]} = 2f\sqrt{1+(x)^{2}} \left(\epsilon_{r}^{0} - \frac{\epsilon_{0}^{0}}{1+(x)^{2}}\right). \qquad 6.4.19$$

If the shell is not closed at the apex, the solution to the differential equation (6.4.19) is

$$u_{r}^{0}(x) = \frac{4f^{2}\rho_{0}}{3E\sqrt{1+(x)^{2}}} \left\{ (1+\nu) \chi \ln \chi - \frac{1+\nu}{\gamma} + \left(\frac{3-\nu}{2}\right) (\chi)^{3} + \frac{1}{4}(\chi)^{5} + C_{1}\left[-(1+\nu) \chi \ln \left(\frac{1+\sqrt{1+(x)^{2}}}{\chi}\right) - \frac{(1+\nu)\sqrt{1+(\chi)^{2}}}{\chi} + \chi\sqrt{1+(\chi)^{2}}\right] + C_{2}\chi \right\}.$$

$$6.4.20$$

The introduction of 6.4.20 into 6.4.18 yields

$$w(\vec{x}) = \frac{4f^{2}\rho_{0}}{3E\sqrt{1+(\vec{x})^{2}}} \left\{ (1+\nu)\left(\ln\vec{x} - \frac{1}{(\vec{x})^{2}}\right) + \left(\frac{3-\nu}{2}\right)(\vec{y})^{2} + \frac{1}{4}(\vec{y})^{4} + \left(1+(\vec{x})^{2}\right) \right\}$$

$$\left[2(\nu-1) + \frac{1+\nu}{(\vec{y})^{2}} + \nu(\vec{y})^{2} + C_{1}\left[-(1+\nu)\ln\left(\frac{1+\sqrt{1+(\vec{x})^{2}}}{\vec{y}}\right) + \sqrt{1+(\vec{y})^{2}}(1+\nu)\right] + C_{2} \right\}.$$

$$6.4.21$$

Several different combinations of boundary conditions will be considered and explicit evaluations as well as numerical calculations will be made. These will be designated as Case 6.4.1.

Case 6.4.2 and Case 6.4.3. The numerical results will be presented in terms of the non-dimensionalized quantities

$$N_r^* = \frac{N_c}{2f\rho_c h}$$
 6.4.22

$$N_{\theta}^{\star} = \frac{N_{\theta}}{2f\rho_{0}h}$$
 6.4.23

$$u_r^* = \frac{u_r E}{4f^2 \rho_0}$$
 6.4.24

$$w^* = \frac{wE}{4f^2\rho_0}$$
6.4.25

The choice of parameters for non-dimensionalization is obvious from equations (6.4.14), (6.4.15), (6.4.20) and (6.4.21).

6.4.1 BOTH EDGES RESTRAINED IN THE TANGENTIAL DIRECTIONS

The boundary conditions for a shell with both edges restrained in the tangential directions are specified by

$$u_{r}^{o}(\gamma_{i})=0,$$
 6.4.1.1

$$u_r^0 (r_2) = 0$$
 6.4.1.2

where Y_1 and Y_2 are the boundaries of the shell (see figure 6.2.1) and are chosen such that

$$r_1 < r_2$$
 6.4.1.3

The solution for \mathbf{C}_1 and \mathbf{C}_2 can be put into the form

$$C_1 = \frac{B(Y_1) Y_2 - B(Y_2) Y_1}{A(Y_1) Y_2 - A(Y_2) Y_1}$$
6.4.1.4

$$c_2 = \frac{B(r_2) A(r_1) - B(r_1) A(r_2)}{A(r_1) r_2 - A(r_2) r_1}$$
6.4.1.5

where

$$A(r_{i}) = r_{i} \sqrt{1 + (r_{i})^{2}} - (1 + \nu) \left[r_{i} \ln \left(\frac{1 + \sqrt{1 + (r_{i})^{2}}}{r_{i}} \right) + \frac{\sqrt{1 + (r_{i})^{2}}}{r_{i}} \right],$$
6.4.1.6

$$B(\gamma_i) = A_1 \left(\frac{1}{\gamma_i} - \gamma_i \ln \gamma_i \right) - A_2 (\gamma_i)^3 - A_3 (\gamma_i)^5$$
.

6.4.1.7

The possibility that the determinant Δ ,

$$\Delta = A(Y_1) Y_2 - A(Y_2) Y_1 \qquad 6.4.1.8$$

in equations (6.4.1.4) and (6.4.1.5) vanishes for some δ_1 and δ_2 should be investigated. Physically this is implausible as it would mean some sort of ins ability. Therefore, one suspects that as a function of δ_2 , the transcendental equation

$$\Delta = 0 \qquad 6.4.1.9$$

for any (fixed) positive δ_1 has no positive roots which are larger than δ_1 . It can indeed be verified that Δ is monotonic and non-zero for all positive $\delta_2 > \delta_1$.

There are six separate configurations which have been analyzed. The results are shown in Figures 6.4.1.1 through 6.4.1.6 which contain curves of N_r^* , N_Θ^* and w^* as well as tables of values. It can

be seen from an examination of the table of values that \mathcal{U}_r^{\bigstar} , the non-dimensional meridional displacement, is an order of magnitude smaller than W^{\bigstar} and it is for this reason that \mathcal{U}_r^{\bigstar} has not been plotted. The boundary conditions that both edges are restrained in the tangential direction means that the shell must deform in the manner shown in Figure 6.4.1.

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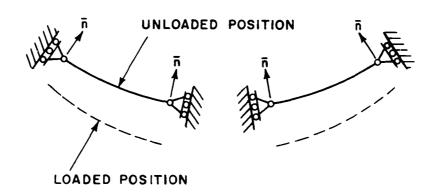


Figure 6.4.1

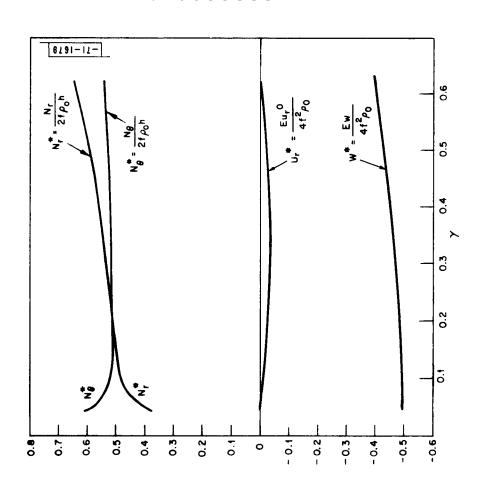
The restrained type of boundary conditions for membranes lead to distortions which at first glance seem peculiar. However, the boundary condition itself, i.e., the restraint of \mathcal{U}_r^* , is somewhat unnatural because it would be very difficult to achieve in a practical situation. Schematically, a set of roller supports as shown in Figure

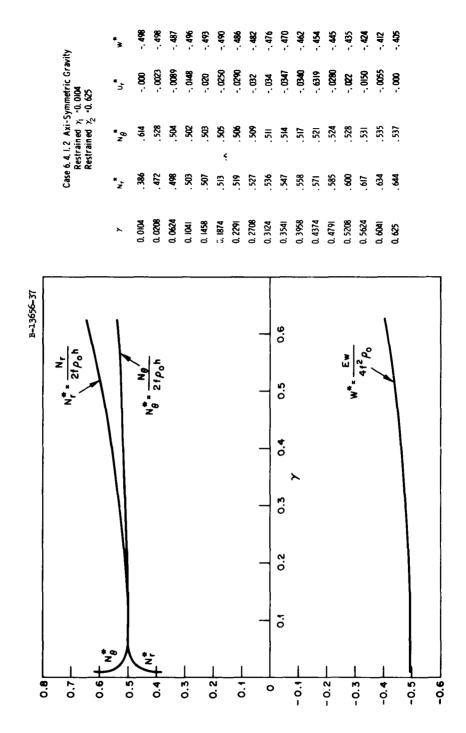
6.4.1 will prevent displacements along a tangent while freely permitting displacements along the normal to the surface of the shell.

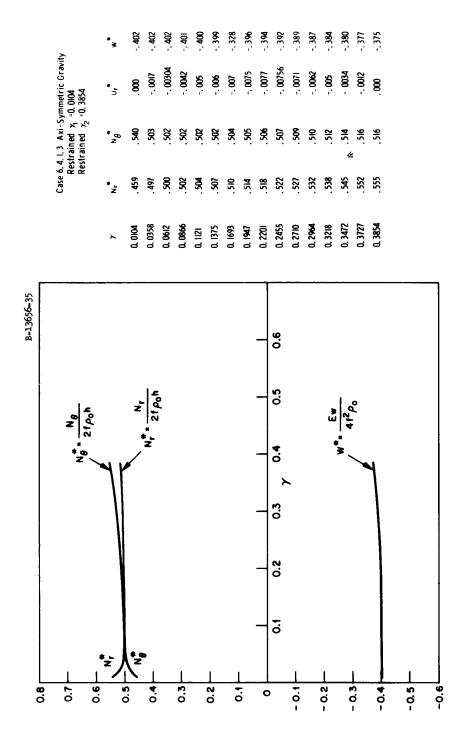
A table which summarizes the cases studied in 6.4.1.1 follows:

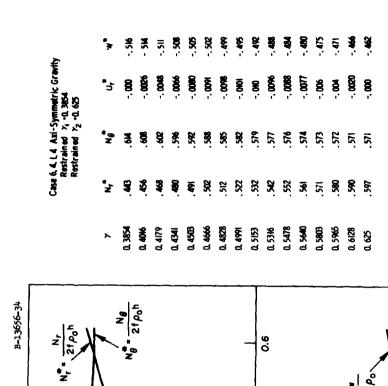
Table 6.4.1.1

Case No.	Restrained at δ_1	Restrained at δ_2
6.4.1.1	0.0446	0.6250
6.4.1.2	0.0104	0.6250
6.4.1.3	0.0104	0.3854
6.4.1.4	0.3854	0.6250
6.4.1.5	०.०५५6	0.8035
6.4.1.6	0.0446	1.0267









0.5

4.0

0.3

0.2

<u>.</u>

0

-0.2

-0.3

4.0

-0.5

-0.6

10.1

>

0.7

0.8

0.6

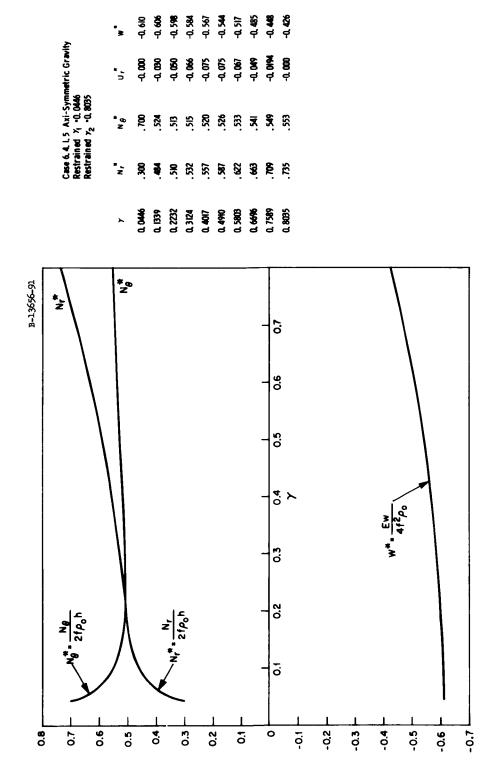
0.5

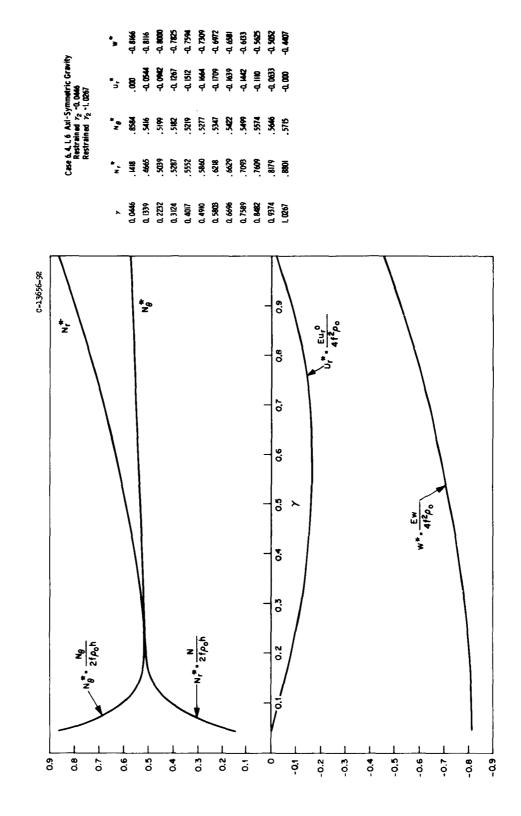
4.0

0.3

0.5

<u>.</u>





6.4.2 ONE EDGE FREE AND ONE EDGE RESTRAINED IN THE TANGENTIAL DIRECTION

The boundary conditions for a shell with one edge free and the other edge restrained in the tangential directions are specified by

$$N_{\Gamma}(r_1) = 0,$$
 6.4.2.1

$$u_r^0 (r_2) = 0$$
 6.4.2.2

where I_1 is the coordinate of the free edge and I_2 is the edge which is restrained. The free edge boundary condition, equation 6.4.2.1, determines the constant C_1 (see equation 6.4.14):

$$C_1 = -[1+(r_1)^2]^{3/2}$$
. 6.4.2.3

The restraint condition, equation 6.4.2.2, enables us to determine the remaining constant (see equation 6.4.20):

$$C_{2} = \left\{ (1+\nu) \left[\frac{\sqrt{1+(y_{2})^{2}}}{(y_{2})^{2}} + \ln \left(\frac{1+\sqrt{1+(y_{2})^{2}}}{y_{2}} \right) \right] - \sqrt{1+(y_{2})^{2}} \right\} C_{1} - (1+\nu) \ln y_{2} + \frac{(1+\nu)}{(y_{2})^{2}} - \frac{(3-\nu)}{2} (y_{2})^{2} - \frac{1}{4} (y_{2})^{4}.$$
6.4.2.4

A total of twenty-nine separate cases have been solved. These represent various combinations of shells with the inner circular boundary unsupported and the outer circular boundary restrained against tangential motion. Additionally, there are cases in which the shell is supported at the inner boundary and is left free at the outer boundary.

The value of N_a^* at the free boundary is seen always to

be

$$N_0^* = 1$$
 6.4.2.5

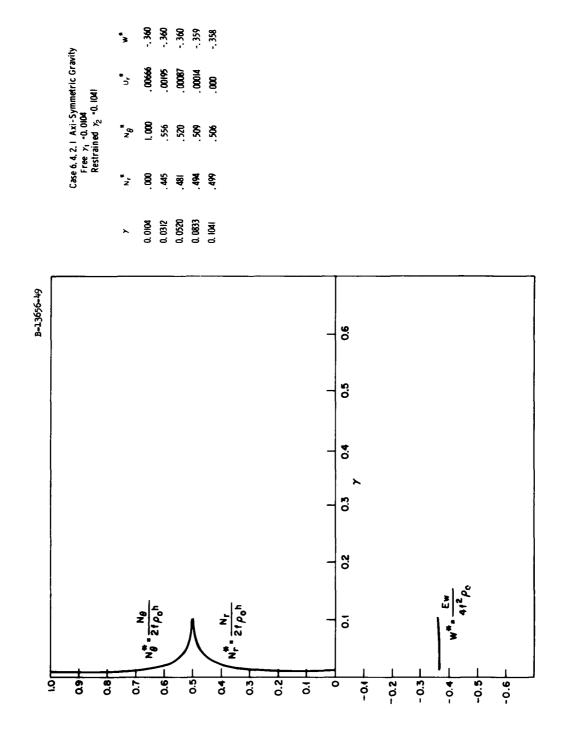
by examining the equilibrium condition, equation 6.4.7, and the loading term, equation 6.4.12.

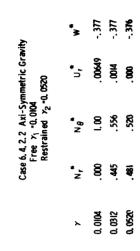
Again, it should be noted that the restrained type of boundary condition will sometimes yield nonsensical results. Thus, case 6.4.2.14, in which the shell is supported at an inner radius of only 8 = .0104, shows non-dimensionalized displacements of over 2000. Obviously, the answer, while mathematically correct, is physically unrealizable. The case has been included as a mathematically interesting example of the effect of a load concentration.

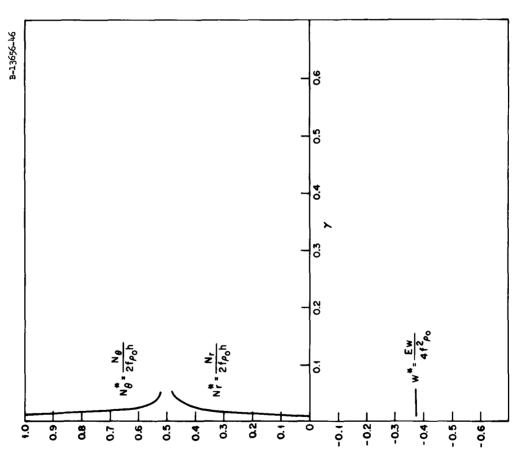
A table which summarizes the cases studied in this section follows:

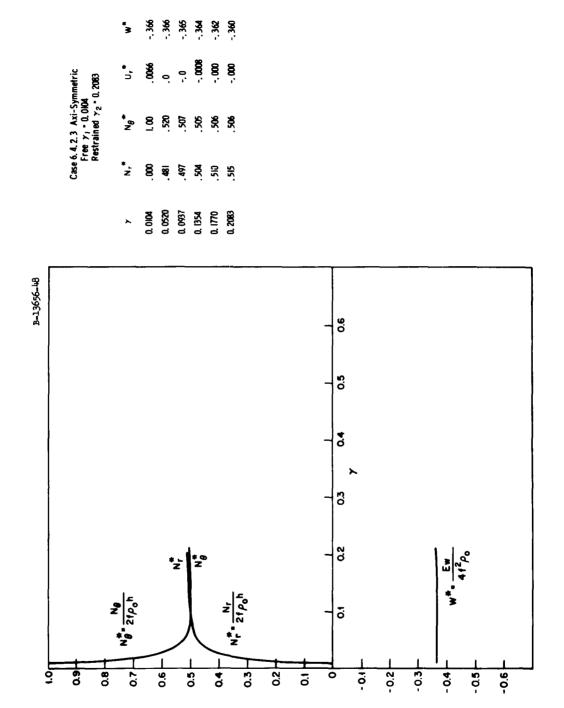
Table 6.4.2.1

Case No.	Free at 8,	Restrained at 8
6.4.2.1	.0104	.1041
6.4.2.2	.0104	.0520
6.4.2.3	.0104	.2083
6.4.2.4	.0104	. 3854
6.4.2.5	.0104	.4687
6.4.2.6	.0446	.4464
6.4.2.7	.0104	.625 0
6.4.2.8	.0223	.625 0
6.4.2.9	.0446	.625 0
6.4.2.10	.0892	.625 0
6.4.2.11	.1785	.625 0
6.4.2.12	.1339	.625 0
6.4.2.13	.4017	.625 0
6.4.2.14	.625 0	.0104
6.4.2.15	.62 50	.0520
6.4.2.16	.625 0	.2083
6.4.2.17	.625 0	. 3854
6.4.2.18	.6250	.4687
6.4.2.19	.0104	.999 0
6.4.2.20	.1785	.8035
6.4.2.21	.11017	.8035
6.4.2.22	.1785	1.0267
6.4.2.23	.4017	1.0267
6.4.2.24	.8020	.2083
6.4.2.25	.8020	. 3854
6.4.2.26	.8020	.4687
6.4.2.27	•9 99 0	.3854
6.4.2.28	1.0000	.4687
6.4.2.29	.0104	.8020









Case 6, 4. 2. 4 Ani-Symmetric Gravity Free γ_1 = 0.0104

Restrained γ_2 = 0.3854

10.0104

0.0104

0.0520

0.0520

0.0530

0.0354

0.170

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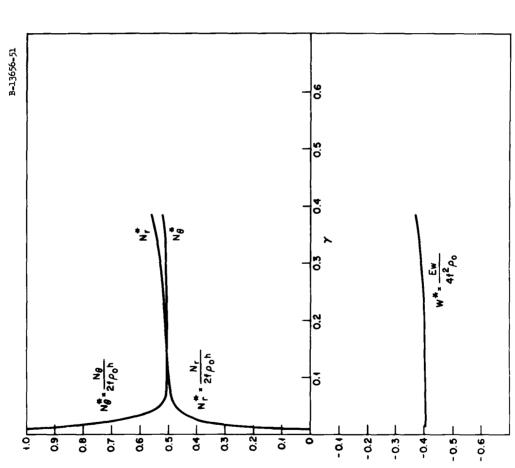
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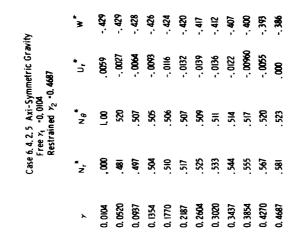
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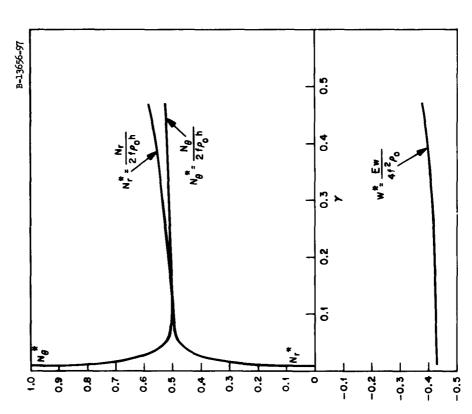
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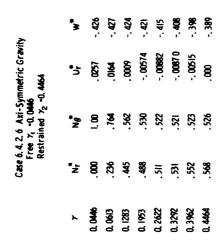
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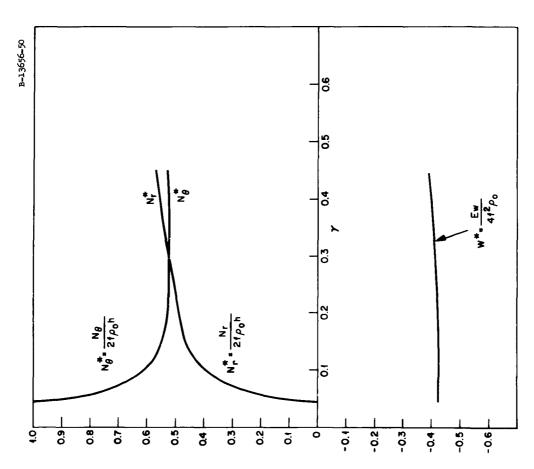
0.3

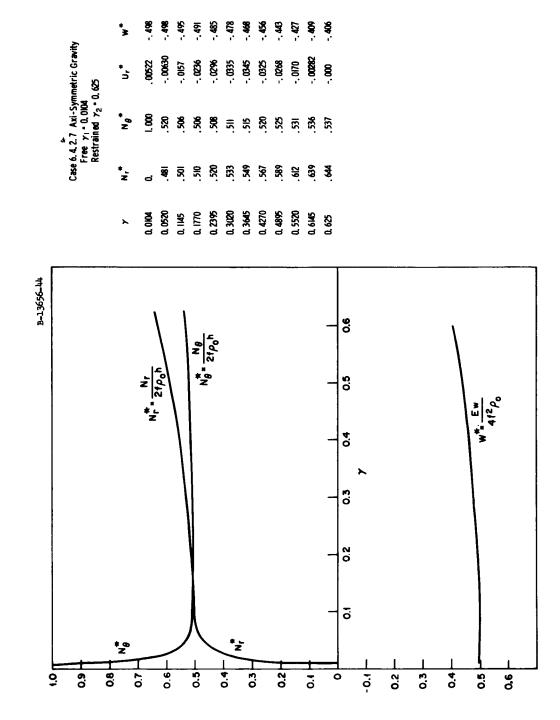






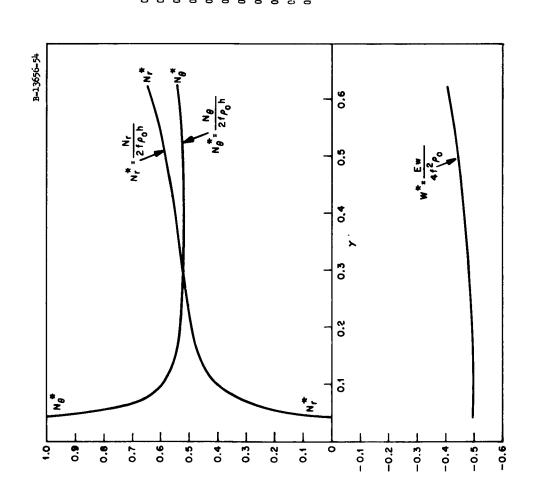


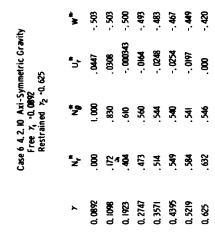


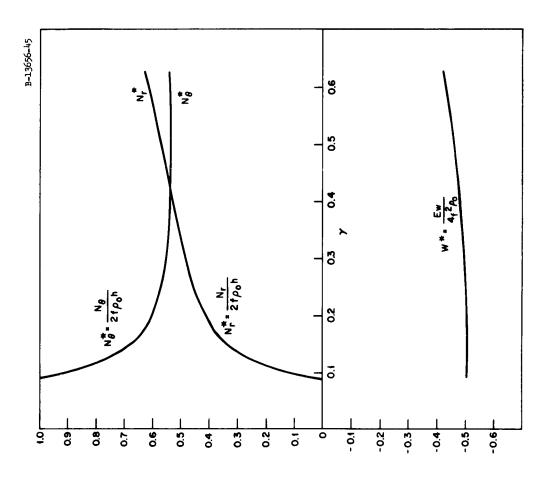


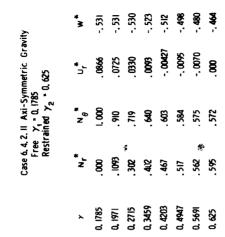
-, 466 -. 416 - 449 -. 439 - 498 . 498 -. 497 -. 495 - 493 -, 489 -. 485 . 480 -. 458 -. 428 C ase 6. 4. 2. 8 Axi-Symmetric Gravity Free γ_1 = 0.0223 Restrained γ_2 = 0.625 - 00686 -. 00827 .00490 -. 0204 - 0340 -. 0243 -. 0174 -. 0143 -. 0254 -. 0294 -. 0323 -. 0325 - 0294 * 0 Z 1,000 1,244 1,544 1,519 1,511 1,511 1,511 1,513 .000 .459 .487 .500 .510 .519 .529 .539 .539 .534 .534 .544 .611 0. 0223 0. 0761 0. 1191 0. 1622 0. 2052 0. 2483 0. 2913 0.3344 0.3774 0.4205 0.4635 0.5066 0.5496 0.5927 B-13656-47 9.0 N8 = 21 Poh W* EW 4420 0.5 Nr = 24 Poh 9. 0.3 0.7 ö 6. F -0.2 -0.3 -0.5 -0.6 9.0 0.5 4.0 0.3 -0.4 4.0 6.0 0.7 0.2 0 0.8

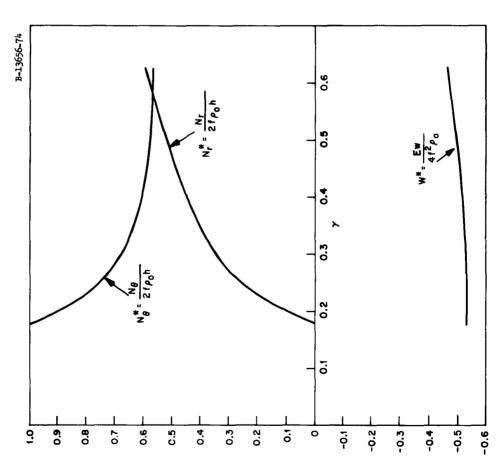
. 488 . 480 - 498 -, 497 - 494 -. 459 - 445 - 428 - 409 Case 6. 4. 2. 9 Axi-Symmetric Gravity Free x₁ · 0. 0446 Restrained x₂ · 0. 625 -. 00392 .0028 - 0220 -. 0302 -. 0306 -. 0165 -. 0322 -. 0250 -. 0149 . 273 . 478 . 503 . 523 . 542 . 563 . 587 0.0446 0.066 0. 1091 0. 1736 0. 2380 0.3025 0.4960 0,4315











Case 6. 4. 2. I. 2 Axi – Symmetric Gravity Free Y₁ = 0.1339

Restrain ed Y₂ = 0.6249

1. Nr Nr Ng* Ur" W**

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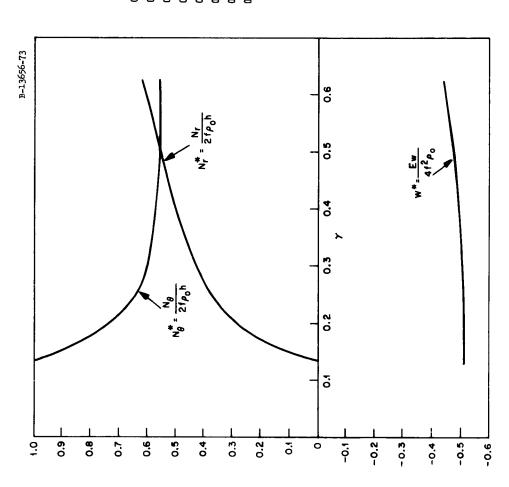
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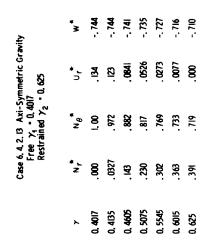
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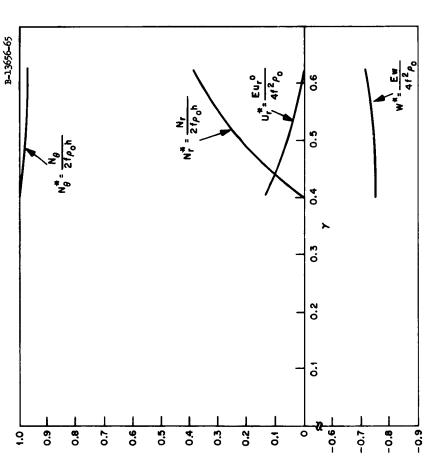
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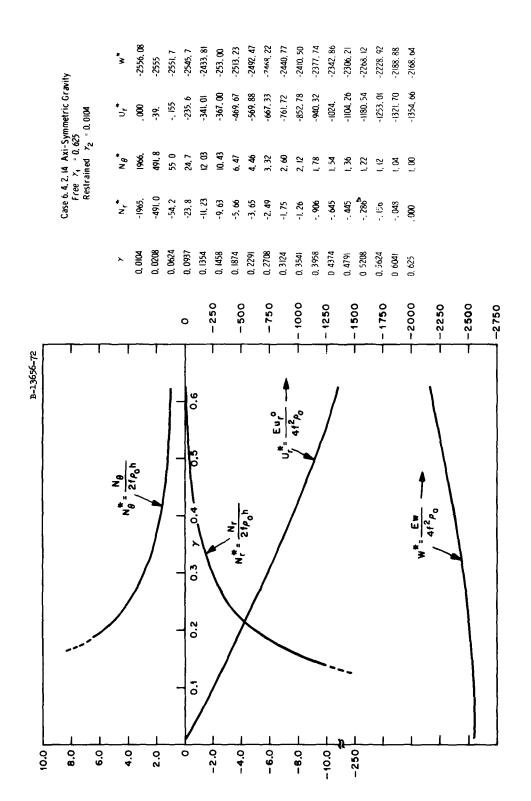
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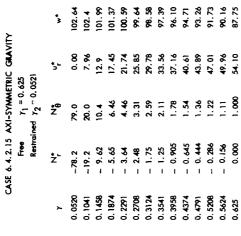
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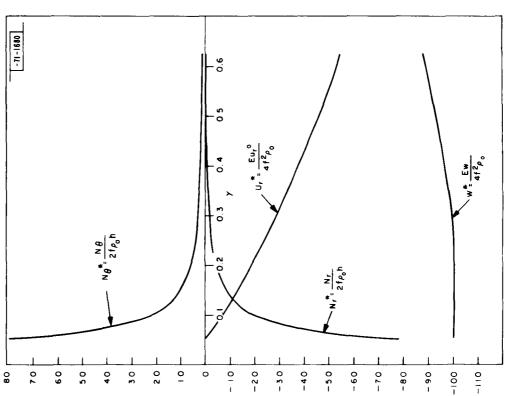






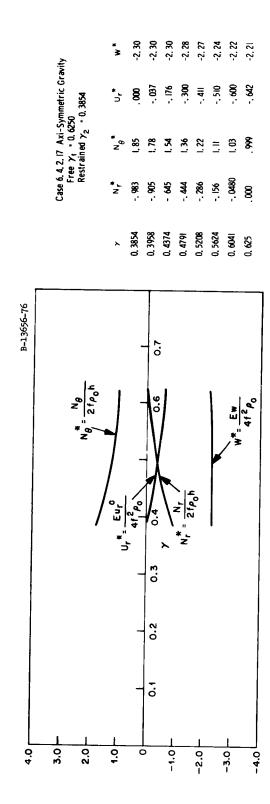


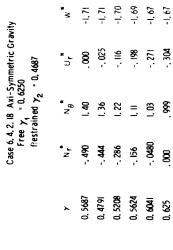


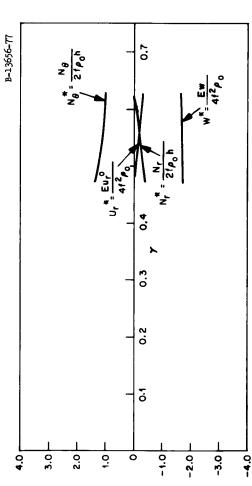


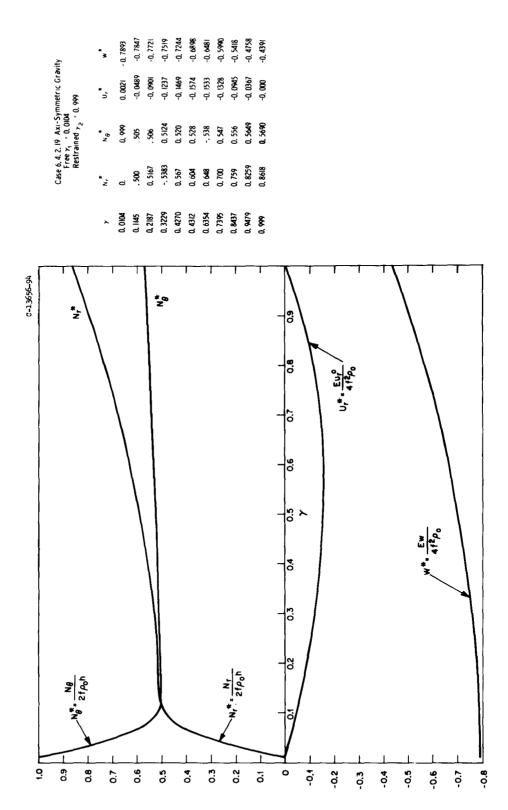
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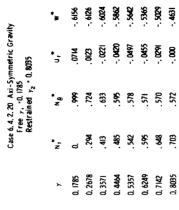
-6.8 -. 679 -6, 76 -6. 72 -6.33 -6.24 -6.16 Case 6, 4, 2, 16 Axi-Symmetric Gravity Free $\gamma_4=0,6250$ Restrained $\gamma_2=0,2083$ -1,352 -2.25 -2.50 -2.74 -2.96 . 98 . 594 -i. 68 -i. 97 .000 4.86 1.59 1.25 1.18 1.05 1.05 3.55 2.74 1.85 -. 0735 -4.50 -4.04 -2.72 ÷.8 -1,36 - 983 -. 703 . 490 -, 322 - 186 0.4270 0.4607 0.5520 0.5937 0.2604 0.3020 0.3437 0,3854 0,5104 0, 2083 0.2187 0.625 в-13656-71 Ng = (Ng) 0.5 Nr = Nr Ur = Euro 4. W* EW 41200 0.3 0.5 9. 0.4 2.0 -4.0 -2.0 -4.0 0 -3.0 -50 -6.0 -7.0 6.0 5.0 3.0 1.0 -8.0

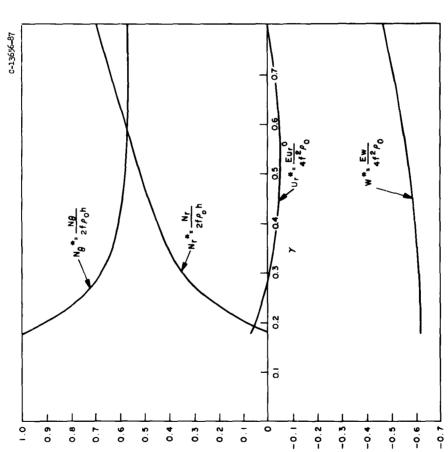


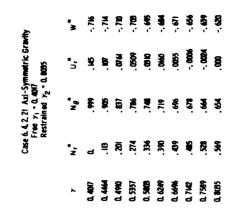


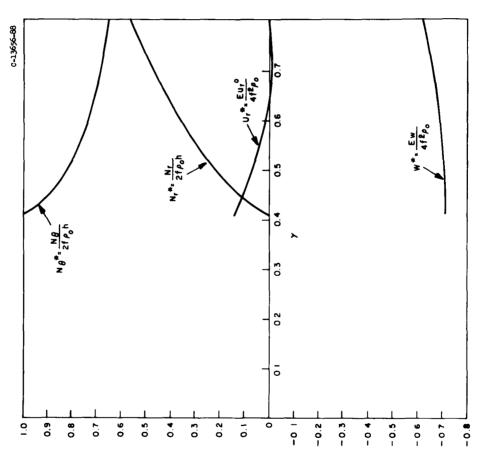


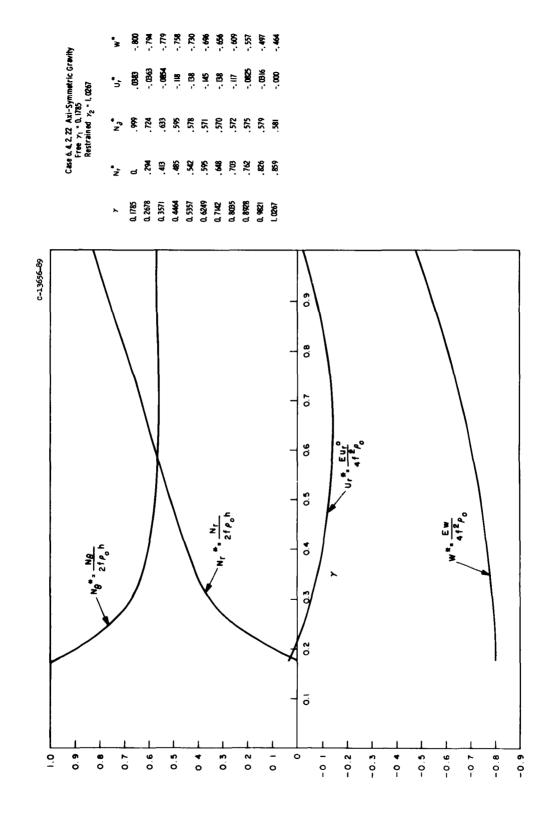


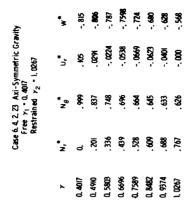


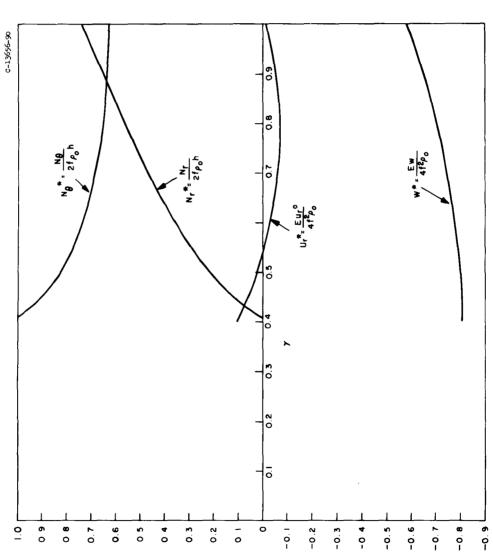


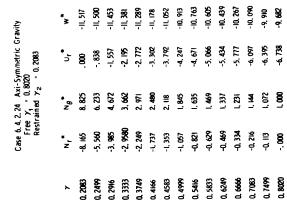


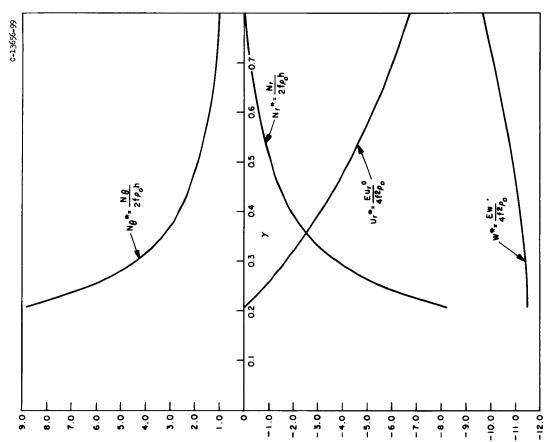


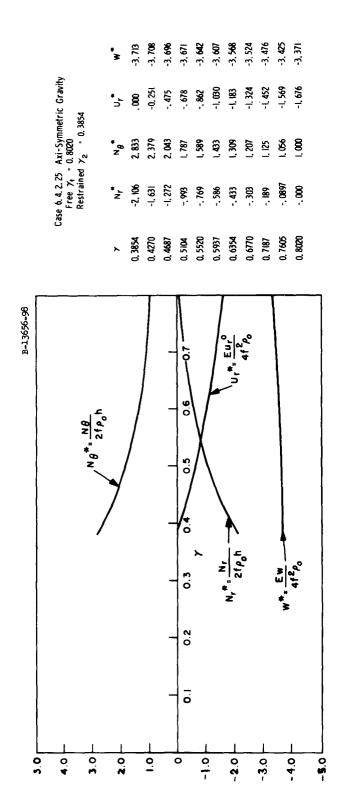


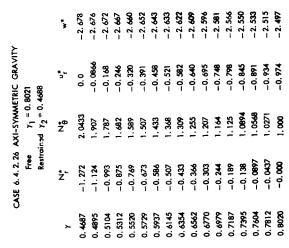


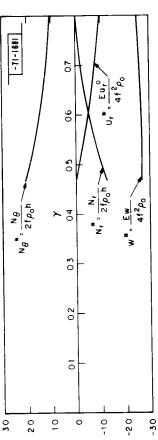


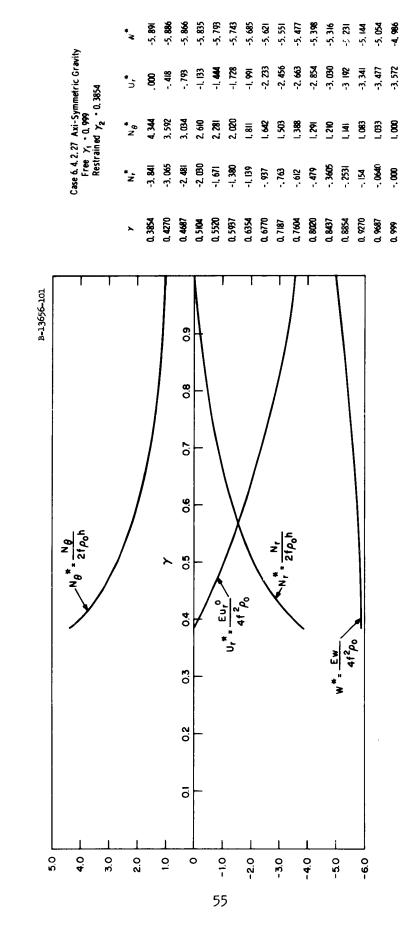




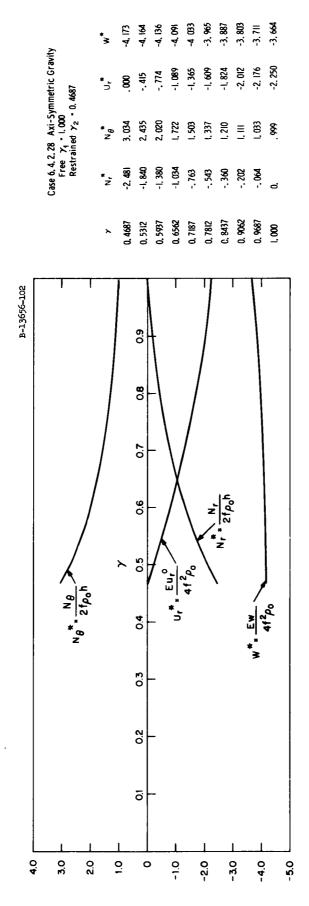


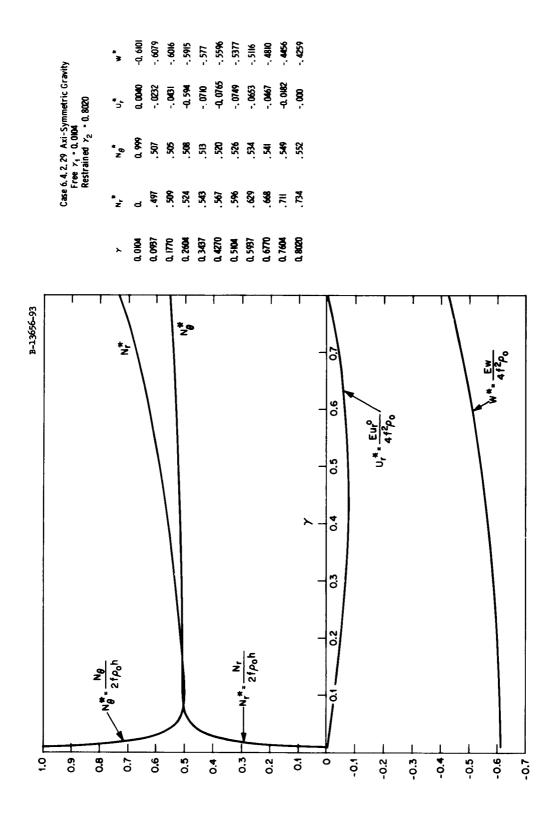






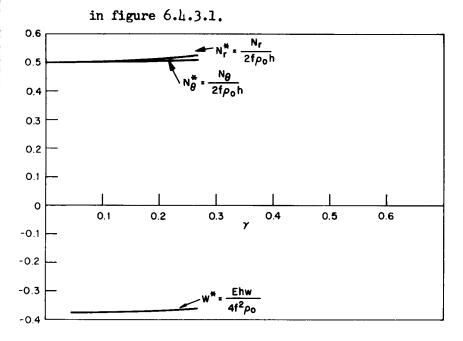
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6.4.3 SHELL CLOSED AT THE APEX

The case in which the shell is closed at the apex is best handled by considering the equilibrium of a portion of the shell bounded by the apex and a circle of latitude (* =constant). This is illustrated



Case 6. 4. 3. 1 Axi	i-Symmetric Gravity
Closed at Ap	ex
Restrained	γ ₂ = 0. 2678

γ	N,*	N _B *	u,*	w*
0	. 500	. 500	0	
0. 0446	. 500	. 500	-0, 0010	-0. 374
0.0669	. 501	. 500	-0, 00 15	-0, 374
0. 0892	. 502	. 500	-0. 0019	-0, 373
0. 1116	. 504	. 501	-0, 0022	-0, 372
0, 1339	. 506	. 502	-0. 002	-0, 371
0. 1562	. 509	. 502	-0. 0025	-0, 370
0. 1785	. 511	. 503	-0. 0024	-0, 369
0. 2008	. 515	. 504	-0, 0021	-0. 368
0. 2232	. 518	. 505	-0, 0016	-0. 366
0, 2455	0, 522	. 507	-0. 0009	-0. 364
0, 2678	. 526	. 508	0.000	-0. 363

Figure 6.4.3.1

The force equilibrium equation of the portion of the shell shown in figure 6.4.3.1 is written as

$$N_{r} \sin \beta \int_{0}^{2\pi} 2f r d\theta = \int_{0}^{2\pi} \int_{0}^{r} 2f \rho_{0} h \sqrt{1 + (r')^{2}} dr' \cdot 2f r' d\theta.$$
6.4.3.1

This results in

$$N_{\Gamma}(r) = \frac{2f\rho_0h}{3} \frac{\sqrt{1+(r)^2}}{(r)^2} \left\{ \left[1+(r)^2 \right]^{3/2} - 1 \right\}.$$

6.4.3.2

At the apex, r=0, equation 6.4.3.2 yields an indeterminate value for N_r (o) since both the numerator and denominator become zero. The application of L'Hospital's Rule to equation 6.4.3.2 yields

$$N_r(0) = f \rho_0 h$$
. 6.4.3.3

If equation 6.4.3.2 is compared to equation 6.4.14 then it is clear that

$$C_1 = -1.$$
 6.4.3.4

If the upper boundary (Γ_2) of the shell is restrained in the tangential direction such that

$$u_r^o(r_2) = 0,$$
 6.4.3.5

then as in case 6.4.2

$$c_{2} = \left\{ (1+\nu) \left[\frac{\sqrt{1+(\gamma_{2})^{2}}}{(\gamma_{2})^{2}} + \ln \left(\frac{1+\sqrt{1+(\zeta_{2})^{2}}}{\gamma_{2}} \right) \right] - \sqrt{1+(\zeta_{2})^{2}} \right\} c_{1}$$

$$- (1+\nu) \ln \zeta_{2} + \frac{(1+\nu)}{(\gamma_{2})^{2}} - \frac{(3-\nu)}{2} (\gamma_{2})^{2} - \frac{1}{4} (\zeta_{2})^{4}.$$

6.4.3.6

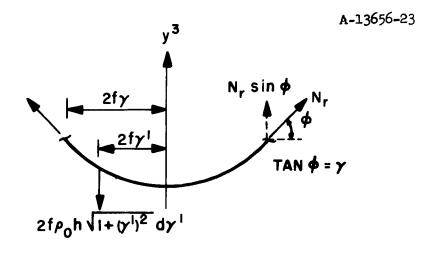
Note that $u_r^0(r)$ will also be finite at $\gamma=0$ by L'Hospital's rule.

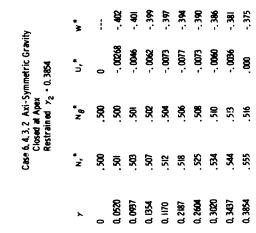
Six cases have been solved for the shell which is closed at the apex. The value of w* at the apex ($\chi=0$) cannot be calculated directly from the solution and hence has not been included. However, it is very easy to extrapolate values of w* near the apex to obtain the value at the apex.

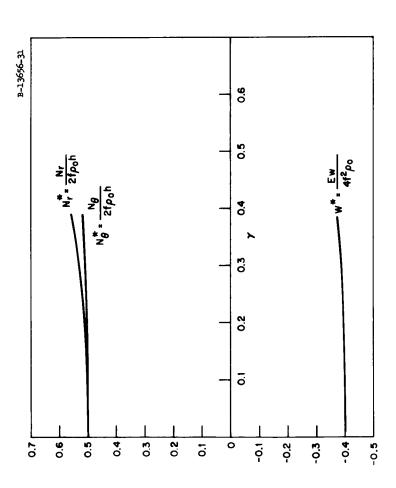
The following table lists the outer boundary of the cases which have been considered.

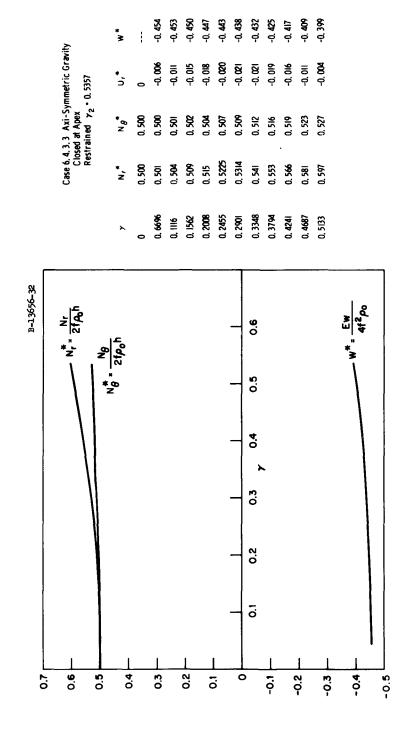
Table 6.4.3.1

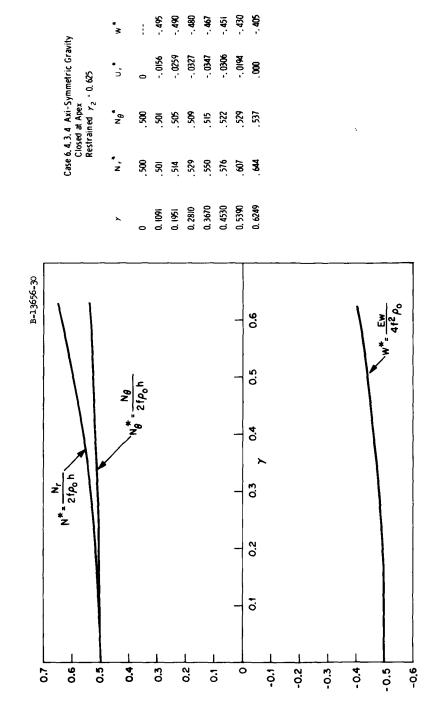
Case	Restrained at	82
6.4.3.1	.2678	
6.4.3.2	.3854	
6.4.3.3	•5 <i>3</i> 57	
6.4.3.4	.625 0	
6.4.3.5	.8035	
6.4.3.6	1.0267	

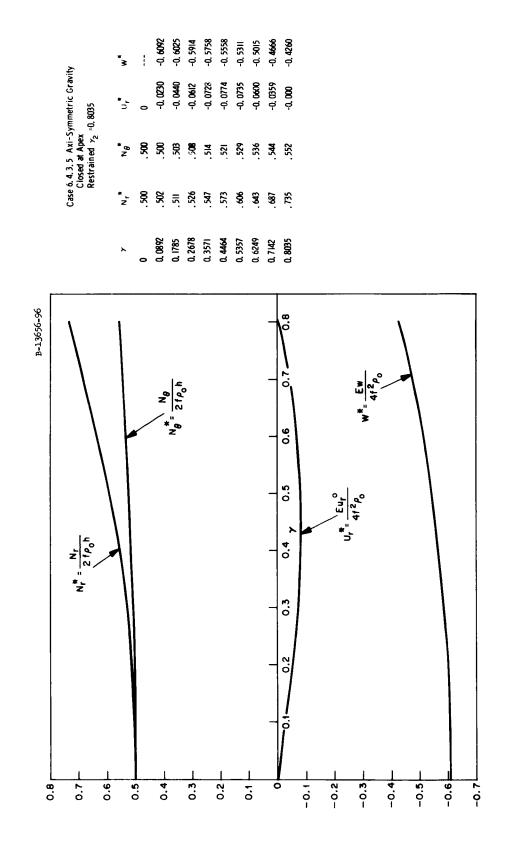


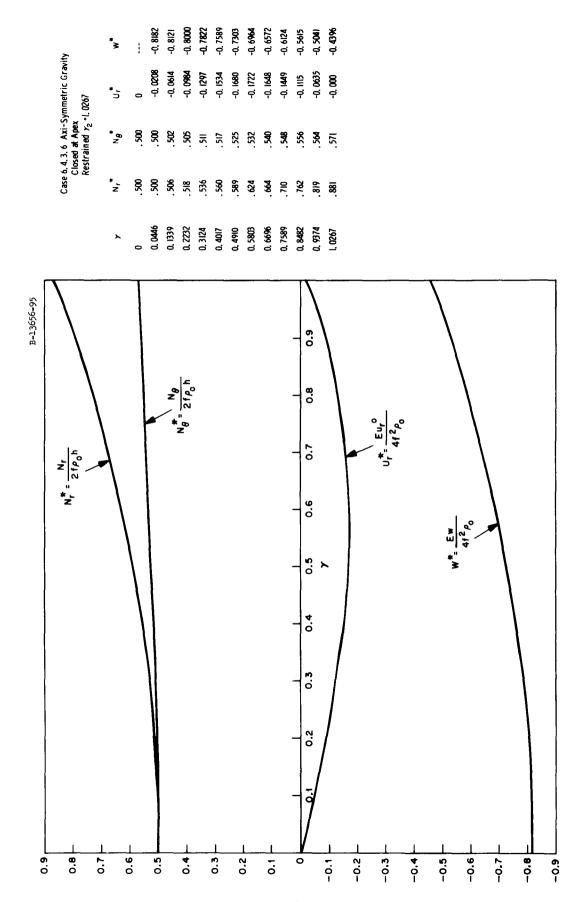












6.5 THE AXI-SYMMETRIC MEMBRANE BEHAVIOR: UNIFORM PRESSURE DISTRIBUTION

If the paraboloidal shell is subjected only to a uniformly distributed pressure q_0 , then the equilibrium equations assume the form (see equations 6.4.6, 6.4.7)

$$Y \frac{dN_r}{dY} + N_r - N_\theta = 0, \qquad 6.5.1$$

$$\frac{N_{\Gamma}}{1+(\Upsilon)^2} + N_{\theta} = -2f \sqrt{1+(\Upsilon)^2} q_0$$
 6.5.2

where it should be noted that p_n has been set equal to q_0 . The differential equation for N_r becomes (see equation 6.4.13)

$$\frac{dN_r}{dy} + N_r \left[\frac{1}{y} + \frac{1}{y \left[1 + (y^2) \right]} \right] = -\frac{2f\sqrt{1 + (y)^2}}{y} q_0. \qquad 6.5.3$$

The solution to this ordinary differential equation is

$$N_r = q_0 f \left\{ -\sqrt{1 + (\gamma)^2} + \frac{\sqrt{1 + (\gamma)^2}}{(\gamma)^2} C_1 \right\}$$
 6.5.4

The hoop force resultant is obtained by substituting

equation 6.5.4 into equation 6.5.2. There results

$$N_{\theta} = -q_{0}f \left\{ \frac{1+2(x)^{2}}{\sqrt{1+(x)^{2}}} + \frac{C_{1}}{(x)^{2}\sqrt{1+(x)^{2}}} \right\}.$$
6.5.5

The expressions for the strains are

$$\epsilon_{\rm r}^{\circ} = \frac{q_0 f}{Eh} \frac{1}{\sqrt{1+(z)^2}} \left\{ -1 + \nu - (2 - \nu) (\gamma)^2 - \frac{1 + \nu + \nu (z)^2}{(z)^2} C_1 \right\}.$$
6.5.6

The differential equation for the meridianal displacements

becomes (see equation 6.4.19)

$$\frac{du_{r}^{o}}{d\vec{x}} - \frac{u_{r}^{o}}{\vec{x}[1+(\vec{x})^{2}]} = \frac{2f^{2}q_{o}}{Eh} \frac{1}{[1+(\vec{x})^{2}]} \left\{ (2\nu-1)(\vec{x})^{4} + 2\nu(\vec{x})^{2} + \left[(\vec{x})^{2} + 2(1+\nu) + \frac{2(1+\nu)}{(\vec{x})^{2}} \right] C_{1} \right\}.$$
6.5.7

The solution for u_r^0 is

$$u_{\Gamma}^{0}(\delta) = \frac{2f^{2}q_{0}}{Eh} \left\{ \frac{(2\nu-1)}{3} (\delta)^{3} + \frac{(2\nu+2)\delta}{3} + C_{1} \left[\Upsilon - \frac{(1+\nu)}{\Gamma} - \frac{(1+\nu)\gamma}{\sqrt{1+(\delta)^{2}}} \right] + \frac{C_{2}\Upsilon}{\sqrt{1+(\delta)^{2}}} \right\},$$

$$(6.5.8)$$

In order to obtain the normal displacement we again make

use of equation 6.4.18. There results

$$W = \frac{2f^{2}q_{0}}{Eh} \left\{ \left(\frac{5-\nu}{3} \right) \left(1+\delta^{2} \right) + C_{1} \left(1+\nu \right) \left[1 - \frac{1}{(1)^{2}} - \frac{1}{\sqrt{1+(1)^{2}}} \right] \right\}$$

$$\ln \left(\frac{1+\sqrt{1+(1)^{2}}}{\delta} \right) + \frac{C_{2}}{\sqrt{1+(1)^{2}}} \right\}.$$
6.5.9

For this case wherein the load is a uniform pressure, the

non-dimensional force-resultants are defined as

$$N_r = \frac{N_r}{2fq_o} , \qquad 6.5.10$$

$$N_{\theta}^{\star} = \frac{N_{\theta}}{2fq_{0}}$$
 6.5.11

and the non-dimensional displacements are defined as

$$u_r = \frac{Ehu_r^0}{4f^2q_0}$$
, 6.5.12

$$W = \frac{EhW}{4f^2q_0}$$
 6.5.13

6.5.1 Case 1 BOTH EDGES RESTRAINED IN THE TANGENTIAL DIRECTIONS

The constants for the case of a shell which is restrained at both edges (see section 6.4.1) are as follows:

$$C_1 = \frac{D(X_1) B(X_2) - D(X_2) B(X_1)}{A(X_1) B(X_2) - A(X_2) B(X_1)},$$
6.5.1.1

$$C_2 = \frac{A(t_1) D(Y_2) - A(Y_2) D(Y_1)}{A(t_1) B(Y_2) - A(Y_2) B(Y_1)}$$
6.5.1.2

where

$$A(\delta_{i}) = \delta_{i} - \frac{(1+\nu)}{\gamma_{i}} - \frac{\gamma_{i}(1+\nu)}{\sqrt{1+(\gamma_{i})^{2}}} \ln \left(\frac{1+\sqrt{1+(\gamma_{i})^{2}}}{\gamma_{i}}\right), \qquad 6.5.1.3$$

$$B(Y_i) = \frac{Y_i}{\sqrt{1+(Y_i)^2}}, \qquad 6.5.1.4$$

$$D(Y_i) = -\frac{(2\nu - 1)}{3} (Y_i)^3 - \frac{(2\nu + 2)}{3} Y_i$$
 6.5.1.5

and Y_1 , Y_2 are the boundaries which are restrained. Again the determinant Δ ,

$$\Delta = A(\xi_1) B(\xi_2) - A(\xi_2) B(\xi_1)$$
does not vanish for all positive ξ_1 , ξ_2 ($\xi_2 > \xi_1$).

6.5.1.6

Six different sizes of shells have been analyzed in this section. It is interesting to note that the largest displacement occurs at the outer boundary. Again, the explanation lies in the restricted type of boundary condition which accompanies membrane behavior (see Section 6.4.1 and Figure 6.4.1). The outer portion of the shell, in order to preserve $U_{\mathbf{r}}=0$ at the boundary, must open up somewhat like a flower in order to accommodate the strains developed by the loads. It is this "opening-up" which induces the larger \mathbf{W}^{\star} displacement.

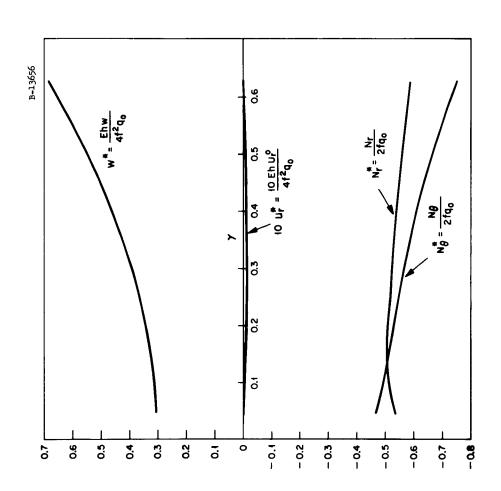
The six cases are summarized in the following table:

Table 6.5.1.1

Case	Restrained at 8,	Restrained at %
6.5.1.1	.0446	.625 0
6.5.1.2	.0104	. 3854
6.5.1.3	. 3854	.6249
6.5.1.4	.0104	. 3854
6.5.1.5	.0446	.80 3 5
6.5.1.6	.0446	1.0267

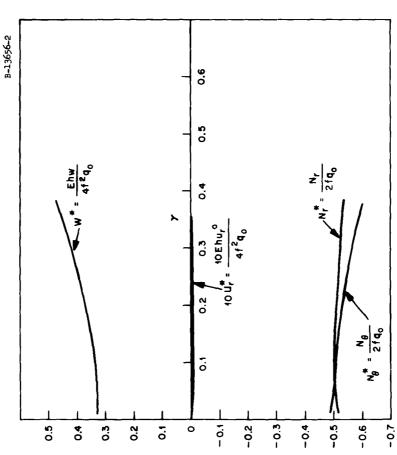
CASE 6.5.1.1 UNIFORM PRESSURE
RESTRAINED $\gamma_{\rm s}^*$.0446
RESTRAINED $\gamma_{\rm s}^*$.625

٧	* L	*60	*"	**
0.0446	- ,536	466	000	305.
0.1306	508	-,508	00506	.320
0.2166	513	533	00810	.350
0.3025	523	565	85600	.395
0.3885	537	909	0093	.454
0.4745	554	655	0073	.526
0.5605	573	710	00366	.610
0.6249	590	-,755	000.	.682



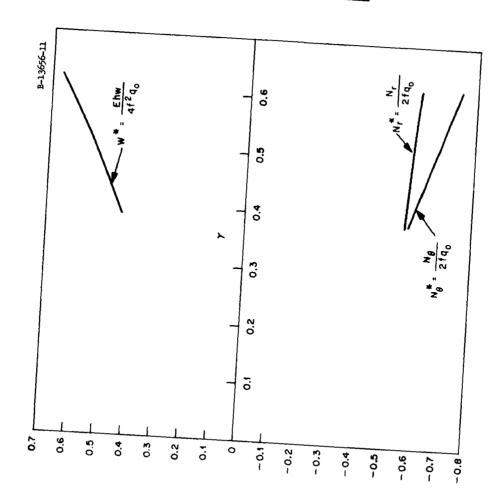
CASE 6.5.1.2 UNIFORM PRESSURE
RESTRAINED 72 = .3854

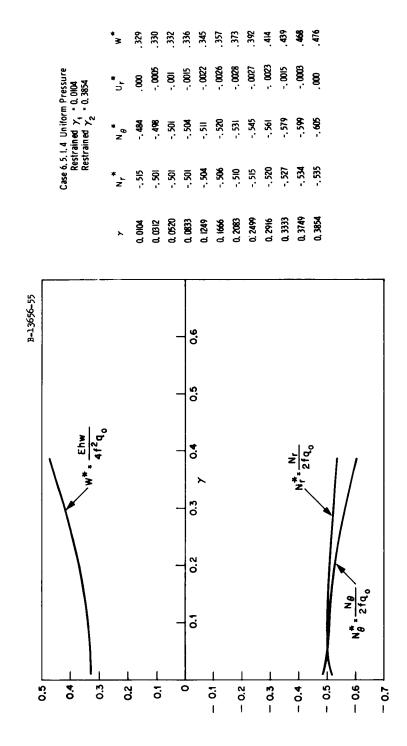
546484 .000501504001005025060017650551400236508523002755125350028751255600287512566002165295640012752956400127	**	.330	.332	.338	.348	.361	.378	765.	.420	.447	.476
- 501 - 501 - 502 - 503 - 508 - 508 - 508 - 508 - 508 - 508 - 552 - 552 - 552 - 552 - 552	*	000	00100	00176	00236	00275	00287	00269	00216	00127	000
	*8	484	504	506	514	523	535	549	566	584	605
	*N	516	501	502	505	508	512	517	522	529	536
0.0104 0.0520 0.0520 0.0537 0.1354 0.1750 0.2187 0.2604 0.3020 0.3437	٧	0.0104	0.0520	0.0937	0.1354	0.1770	0.2187	0.2604	0.3020	0.3437	0.3854

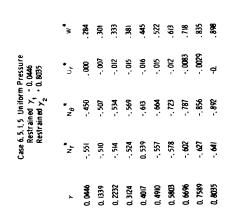


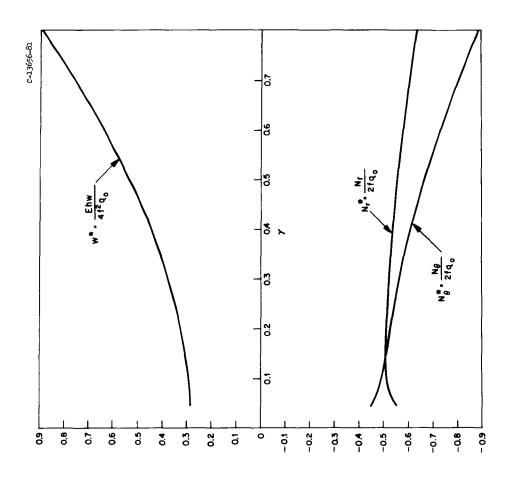
CASE 6.5.1.3 UNIFORM PRESSURE RESTRAINED 7:=.3854 RESTRAINED 7:=.6249

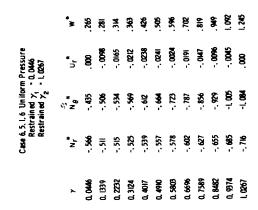
**	437	470	905	.544	.586	.631	999.	
*>	000.	00136	00206	00217	00177	806000	000	
* 0	578	606	634	662	692	722	746	
* ~	567	569	574	580	- ,587	- 596 -	603	\dashv
>	0.3854	0.4270	0.4687	0.5404			0.6249	 1

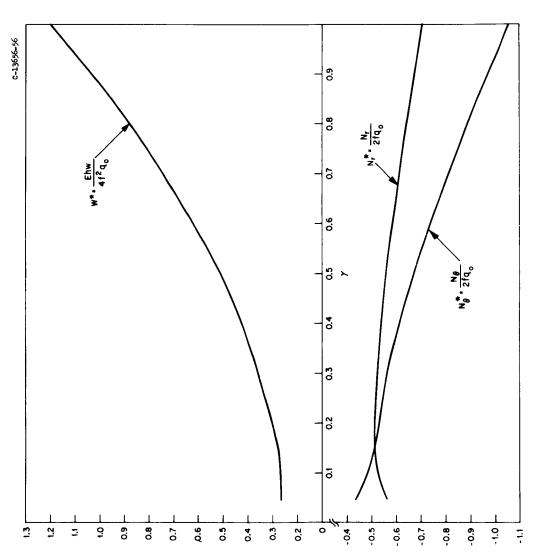












6.5.2 ONE EDGE FREE AND ONE EDGE RESTRAINED IN THE TANGENTIAL DIRECTION

We will specify the free edge to be 7_1 , and 7_2 to be the restrained edge (see section 6.4.2). Then the constants are as follows:

$$C_1 = (x_1)^2$$
, 6.5.2.1

$$C_{2} = \frac{\sqrt{1 + (\chi_{2})^{2}}}{\chi_{2}} \left\{ \left[\frac{(1 + \nu)\chi_{2}}{\sqrt{1 + (\chi_{2})^{2}}} \ln \left(\frac{1 + \sqrt{1 + (\chi_{2})^{2}}}{\chi_{2}} \right) + \frac{(1 + \nu)}{\chi_{2}} - \chi_{2} \right] C_{1} - \left[\frac{(2\nu - 1)}{3} (\chi_{2})^{3} + \frac{2(1 + \nu)}{3} \chi_{2} \right] \right\}.$$

$$6.5.2.2$$

Twenty-three different size shells have been analyzed in this section. A small hole, while affecting the distribution of N_r^* and N_θ^* , does not significantly affect the displacement W^* (compare cases 6.4.1.1 and 6.5.2.5). The discussion of W^* in the previous section also applies for the cases in this section where the free boundary is the inner radius. For the cases where the shell is supported at the inner radius and is free at the outer boundary (see e.g., case 6.5.2.12), the W^* displacements are larger at the support. Again, this is somewhat contrary to one's intuition. The explanation is similar to that advanced in the previous sections; the larger strains generated at the supported boundary can be accommodated with the restriction of $u_r = 0$ only by large values

of the normal displacement. Note also (see case 6.4.2.12) that the tangential displacement becomes appreciable if the free boundary is the outer radius of the shell.

A list summarizing the cases which have been studied is contained in Table 6.5.2.1.

Table 6.5.2.1

Case	Free at 8,	Restrained at χ_2
6.5.2.1	.0223	.6 249
6.5.2.2	.0892	.625 0
6.5.2.3	.1339	. 6250
6.5.2.4	.1785	. 6 2 50
6.5.2.5	.0446	.6250
6.5.2.6	.0104	.3854
6.5.2.7	.625 0	.3854
6.5.2.8	.1785	.8035
6.5.2.9	.4017	.8035
6.5.2.10	.1785	1.0267
6.5.2.11	.4017	1.0267
6.5.2.12	.8020	.2083
6.5.2.13	.8020	. 3854
6.5.2.14	.8020	.4687
6.5.2.15	·9 99 0	.3854
6.5.2.16	1.0000	.4687
6.5.2.17	.0104	.1041
6.5.2.18	.625 0	.2083
6.5.2.19	.0104	.2083
6.5.2.20	.0104	.4687
6.5.2.21	.0104	.8020
6.5.2.22	.0104	• 999 0
6.5.2.23	.6250	.0520

305 322 354 .402 .465 545 .633 .683

- .010

541 .573

-.507

.2232 .3124 4017

-.521

1.011

- .008

- .527

-,490

.1339

-.0027 -.0071 - .0098

- .724

-.577

5803 6249

999

-.556

.4910

- .615

-.537

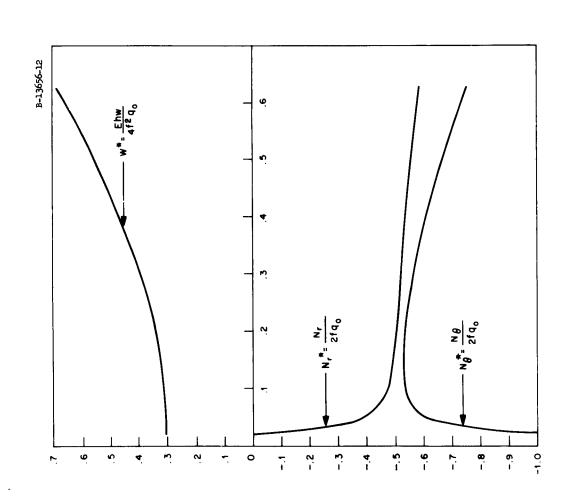
000

.756

-.586

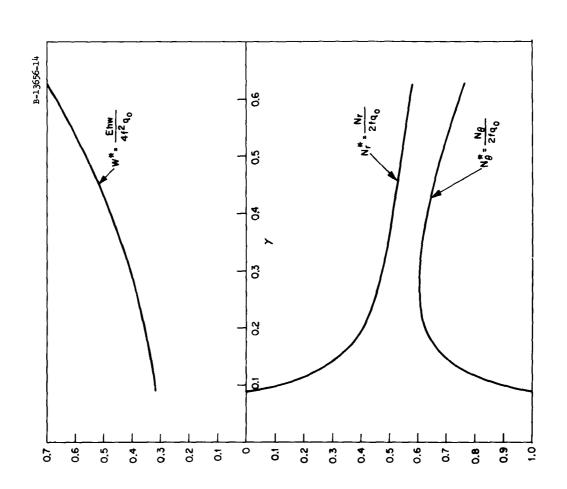
304 * CASE 6.5.2.1 UNIFORM PRESSURE -.0156 - .009 FREE y₁ = .0223 RESTRAINED y₂ = .6249 *_ - .626 -1.00 *₀ * _ Z -.000 -.375 .0223 .0446

>

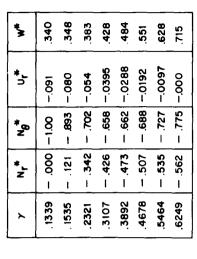


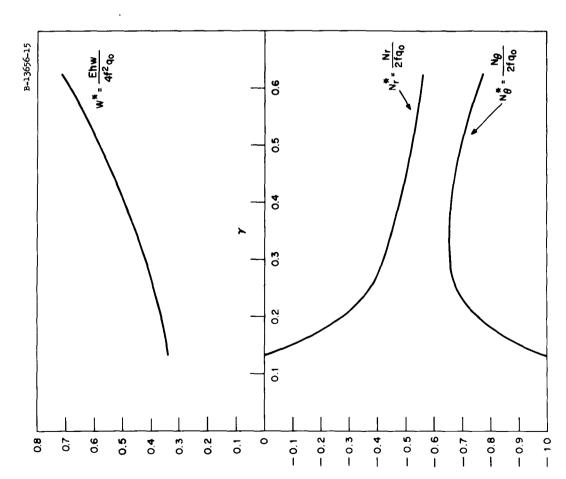
CASE 6.5.2.2 UNIFORM PRESSURE FREE y =.0892 RESTRAINED y =.625

**	.318	.323	.35(.391	444	509	.585	.650	969.
U,	0616	0514	€.033	-,0255	0204	0147	0087	0036	000
*N	- 1.00	837	633	606	620	~.653	869	736	764
*L	000	171	399	464	- 498	524	-,548	565	578
٨	2680.	8601.	1923	.2747	.3571	.4345	.5219	.5837	.6249



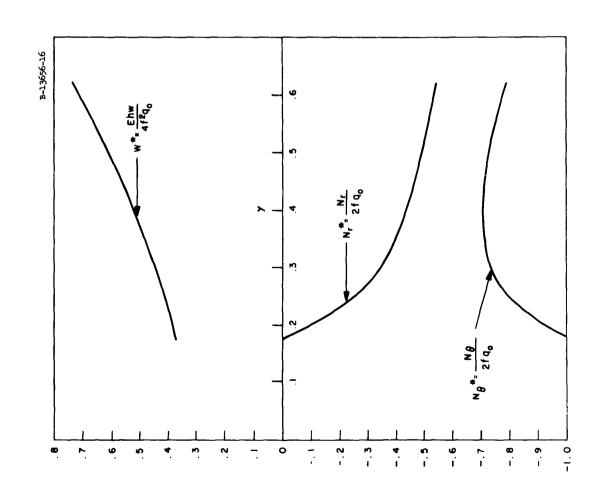
CASE 6.5.2.3 UNIFORM PRESSURE FREE γ = .1339 RESTRAINED γ = .625





CASE 6.5.2.4 UNIFORM PRESSURE FREE γ =.1785 RESTRAINED γ =.625

٨	* "X	* ⁸ N	*0	**
1785	000'-	-1.016	211'-	.375
1.197.1	9160	93!	901	.384
.2715	294	762	074	. 425
.3459	388	712	0536	. 475
.4203	444	707	037	. 533
.4947	485	726	023	· 601
1695.	519	759	7600	.677
.6249	541	061	000	.740



Case 6. 5. 2. 5 Uniform Pressure
Free 7₁ = 0. nd46

Restrain of 7₂ = 0. 625

0. 0446

0. 0446

0. 0466

0. 0561

0. 573

0. 573

0. 580

0. 584

0. 480

0. 554

0. 480

0. 554

0. 480

0. 554

0. 480

0. 554

0. 480

0. 584

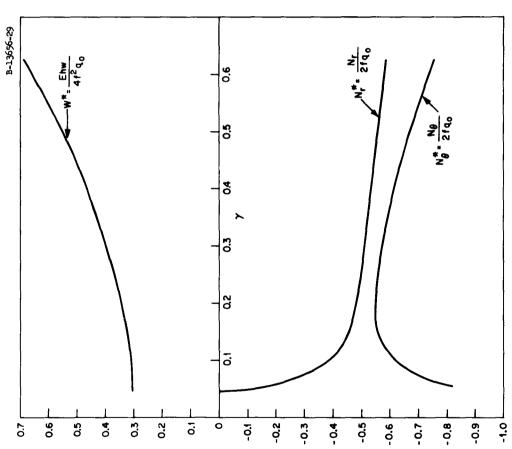
0. 6249

0. 587

0. 777

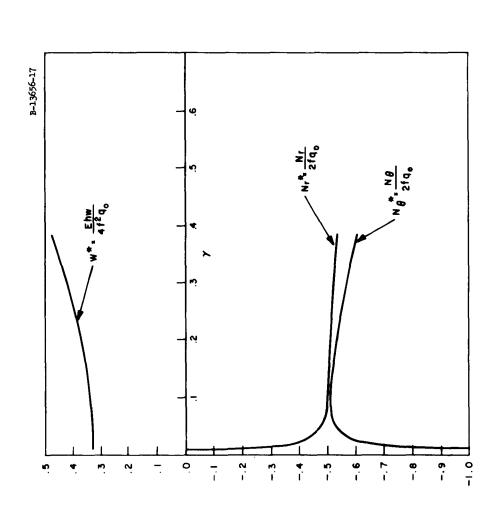
0. 628

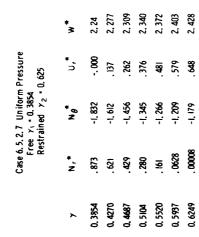
0. 686

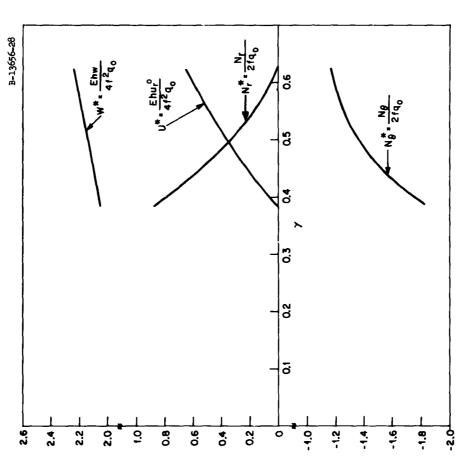


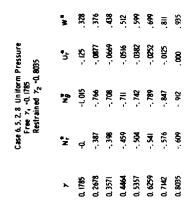
CASE 6.5.2.6 UNIFORM PRESSURE FREE 7 = .0104 RESTRAINED 7 = .3854

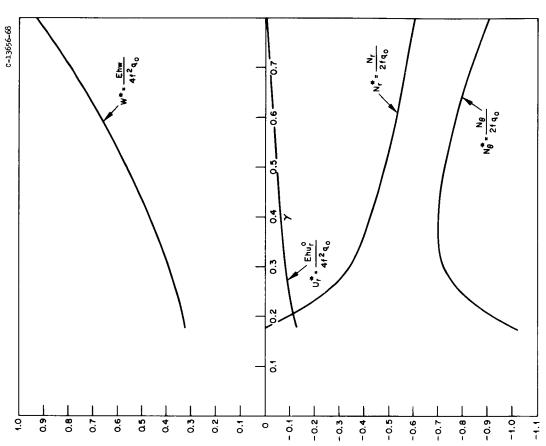
^	*. 2	* 8 N	* 0	**
4010.	000	-1.00	00698	330
.0208	375	625	00380	.330
.0624	487	715	00234	.334
104.	498	513	00259	34
. 1458	503	816	00292	.351
.1874	507	528	00311	365
1622.	512	540	00306	.383
.2708	517	554	00272	.403
.3124	523	175	00205	. 427
.3541	530	065	00102	454
.3854	535	605	000	.477

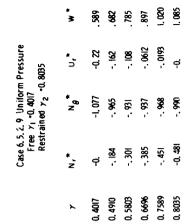


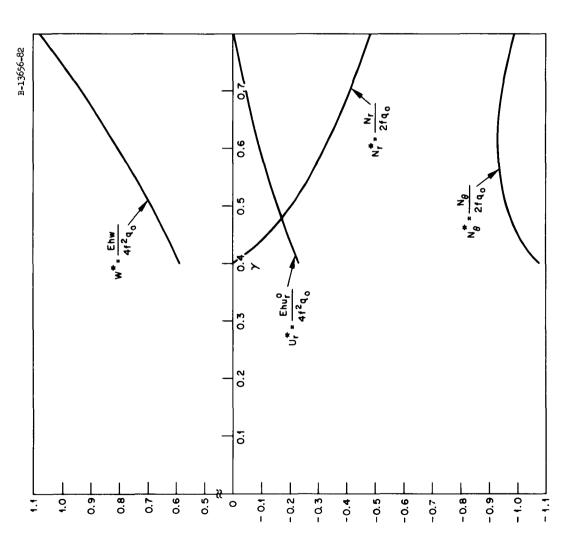


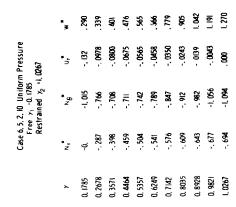


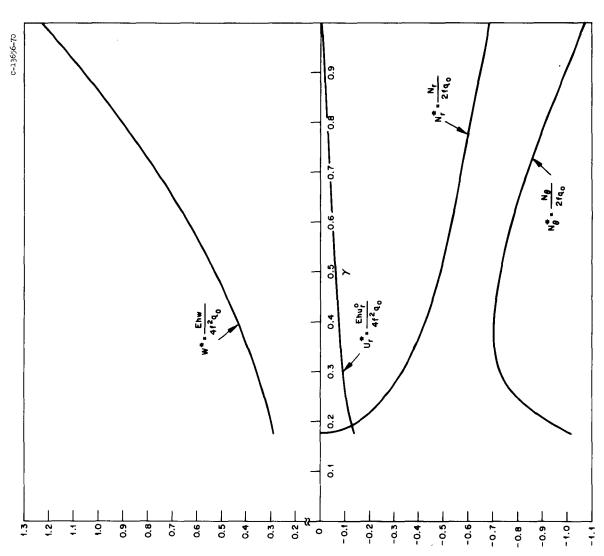


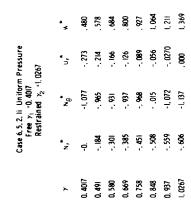


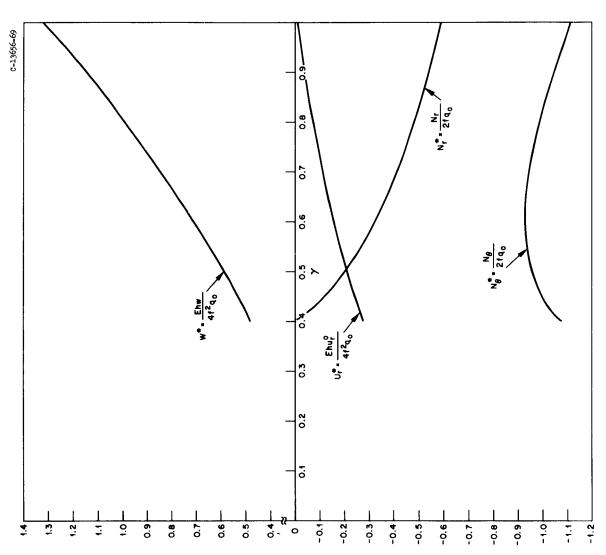


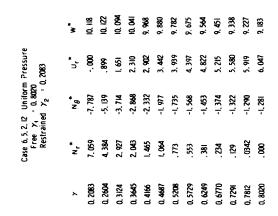


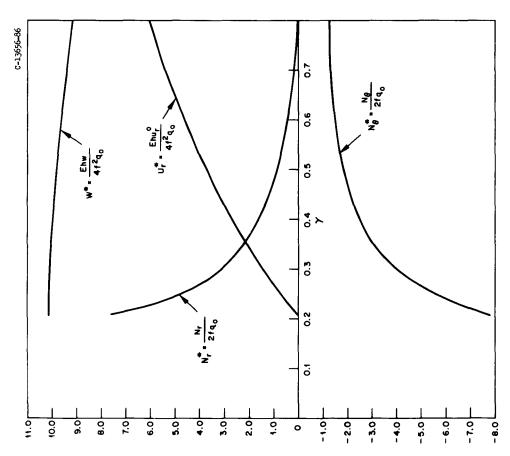


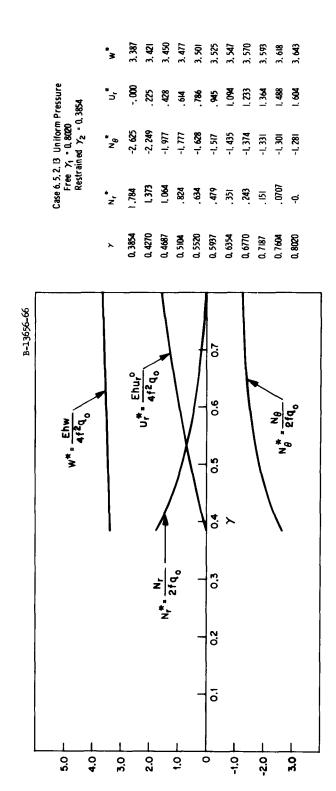




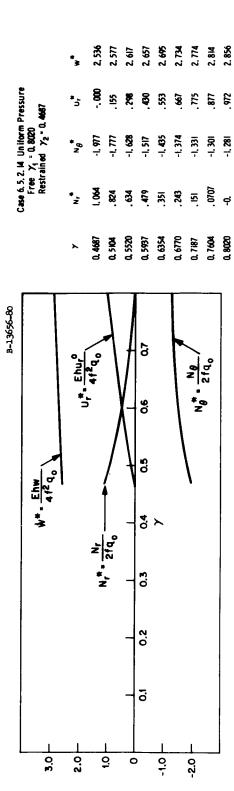


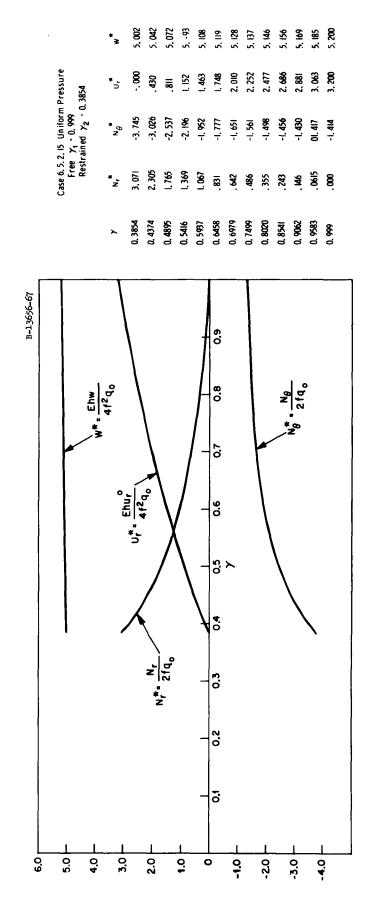


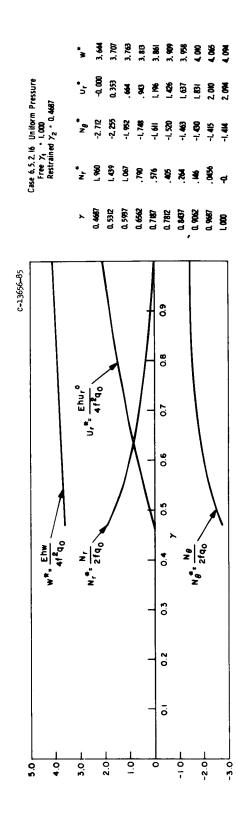


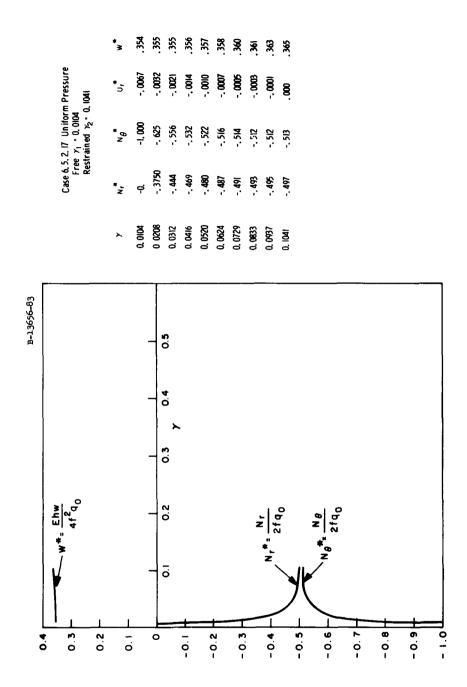


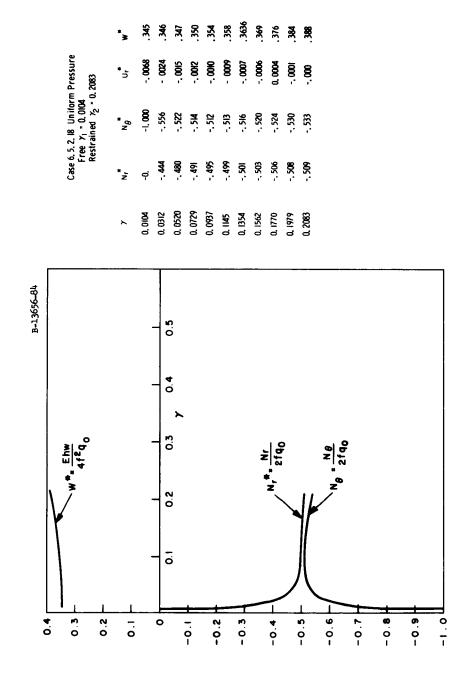
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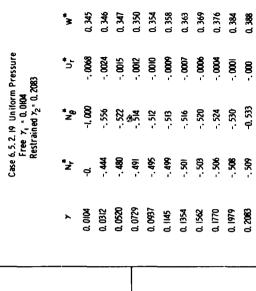


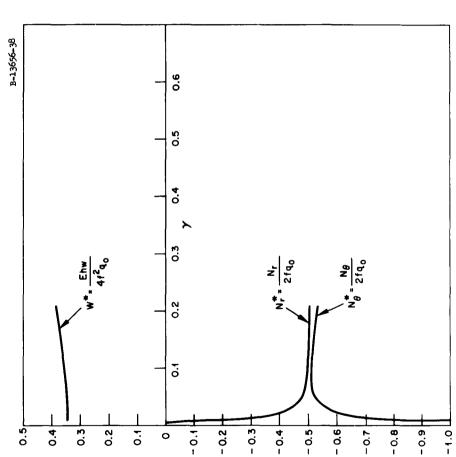


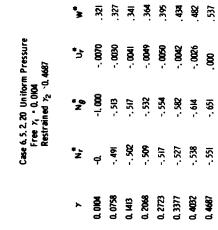


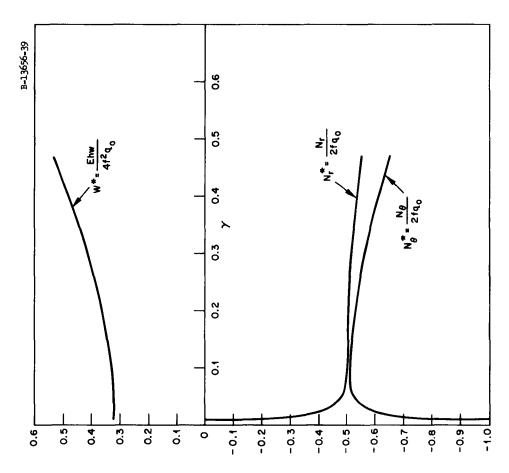


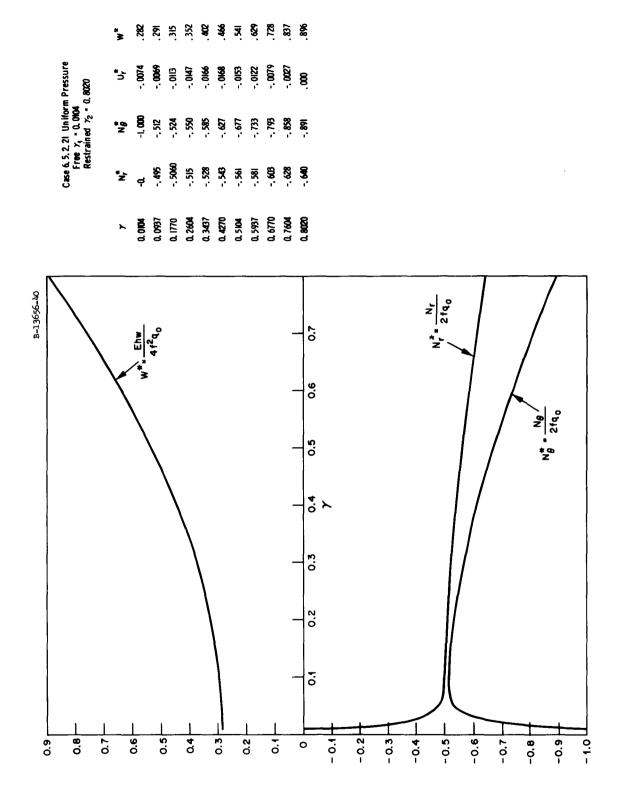




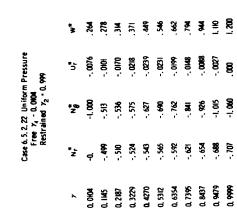


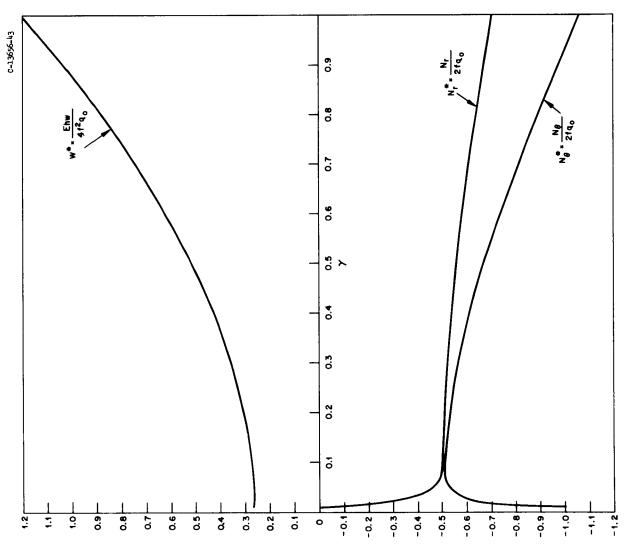




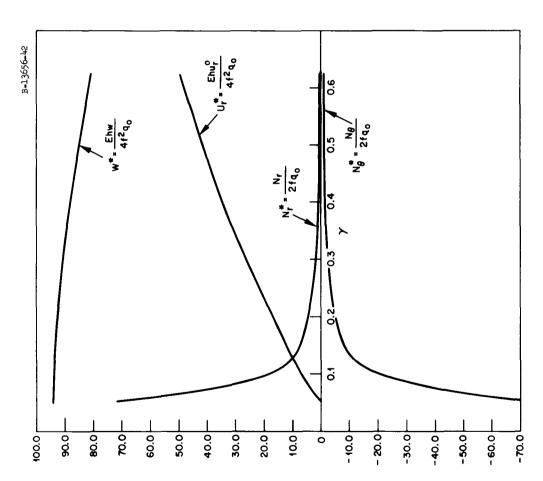


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	*	SE 010	93.312	91, 574	89, 063	85.998	82.586	30, 306
Pressure 625	*ວັ	.000	12.887	22, 751	31. 584	39, 491	46, 477	49, 634
Case 6, 5, 2, 23 Uniform Pressure Free 7, -0, 0520 Restrained 7 ₂ -0, 625	*®	-72.404	- 8. 422	- 3.336	- 1. 975	· I. 456	-1.234	- 1, 179
Case 6. 5. 2 Free Restr	**	71, 596	7.59	2, 459	1.031	.429	<u>6</u> 0.	900.
	٨.	0.0520	0, 1562	0, 2604	0,3645	0.4687	0, 5729	0.625



6.5.3 SHELL CLOSED AT THE APEX

If the shell is closed at the apex, i.e., the shell "begins" at l=0, then in order for the force resultants to remain finite at l=0 (see equation 6.5.4), we must have

$$C_1 = 0.$$
 6.5.3.1

Then, if the edge, ℓ_2 , is restrained against tangential displacement, the constant C_2 is given by the expression

$$C_2 = -\frac{\sqrt{1+(\gamma_2)^2}}{3} \left[(2 \nu - 1) (\gamma_2)^2 + 2 (\nu + 1) \right]. \qquad 6.5.3.2$$

Six different sizes of shells have been analyzed. It is interesting to note that the presence of a moderate size hole at the apex of a shell has very little influence on the normal displacements even though the distributions of the force resultants are appreciably different (compare, e.g., cases 6.5.1.1, 6.5.2.5, 6.5.3.1).

A list of the cases which have been analyzed is contained in Table 6.5.3.1

Table 6.5.3.1

Case	Restrained at	82
6.5.3.1	.6250	
6.5.3.2	.3854	
6.5.3.3	•5 3 57	
6.5.3.4	.2678	
6.5.3.5	.8035	
6.5.3.6	1.0267	

CASE 6.5.3.1 UNIFORM PRESSURE CLOSED AT APEX.
RESTRAINED 7 = .625

B-13656-7

0.6

0.7

0.5

9.

03

0.2

ö

0

1.0-

-0.5

-0.3

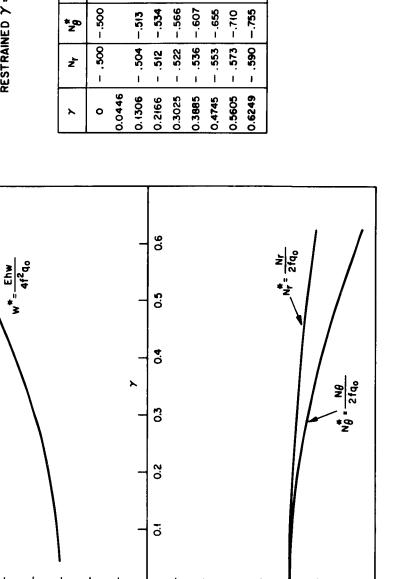
4.0-

-0.5

-0.6

-0.7

-0.8



.320

-.0058

.351

-.00**85** -.0098

305

-.002

*

*ა

.454

-.0095

.610

-.0037

000

395

ASE 6.5.3.2 UNIFORM PRESSURE CLOSED AT APEX.
RESTRAINED y = .3854

B-13656-6

.332 .338 .348 .361 .361 .378 .397 .420

-.000213

0

8

*≥

*5

*0

*Ľ

- () (2																
CASE	٨	0	0.0104	0.0520	0.0937	0.1354	0.1770	0.2187	0.2604	0.3020	0.3437	0.3854				
			0.6													
		_	0.5						*2	0412		į	ν* = 2fqo			
w*= Ehw 4f ² q ₀		-	0.4						*2	1	l .	مما	*0 /			
\	,		0.3													
			0.2													
		Ī	0.1													
,), ,	1		0	ı	ļ		ı	1]			ı	1	1	ı	
0 0 0 4 % 5	<u>.</u>		L	-0.4	- C	;	- 0.3	4	†	0.5	0	Ф О	-07	1 0.8	6.0	- 4.0

-.00288

- .512

-.00238

-.00179

- .514 - .514 - .523 - .535 - .549 - .566

- .502 - .504

-.00104

-.502

- 501

-.00269

-.00246 -.00427

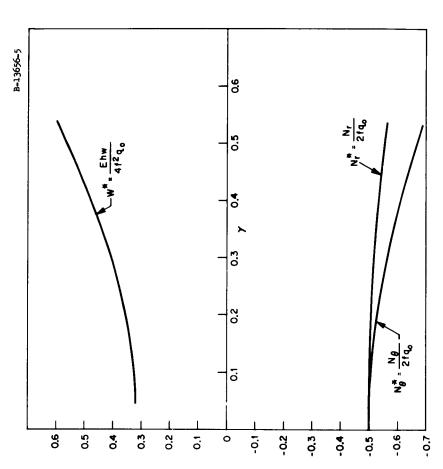
- .522

000

-.605

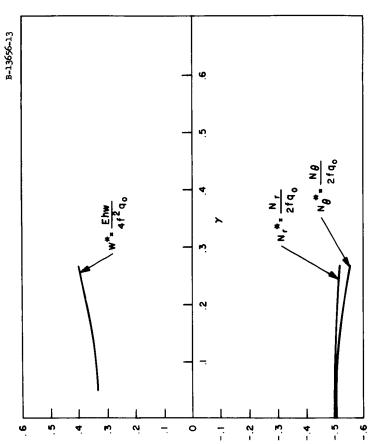
CASE 6.5.3.3 UNIFORM PRESSURE CLOSED AT APEX RESTRAINED γ_2 =0.5357

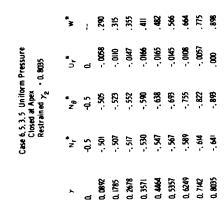
		b		k
	-0.500	-0.500	0	
0.0669	-0.501	-0.503	-0.002	-0.318
0.1116	-0.503	-0.509	-0.003	-0.326
0.1562	-0.506	-0.518	-0.005	-0.338
0.2008	-0.509	-0.529	-0.0060	-0.354
0.2455 -	-0.514	-0.544	-0.0066	-0.374
0.2901	- 0.520	-0.561	-0.0068	-0.398
0.3348	- 0.527	-0.580	-0.0066	-0.425
0.3794	-0.534	-0.602	-0.0059	-0.456
0.4241	-0.543	-0.625	- 0.004	-0.491
0.4687	-0.552	-0.651	-0.003	-0.529
0.513	-0.562	-0.679	-0.001	-0.571

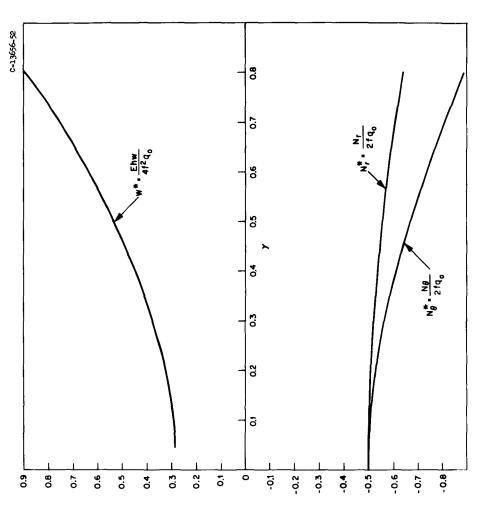


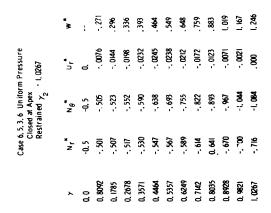
CASE 6.5.3.4 UNIFORM PRESSURE CLOSED AT APEX RESTRAINED 7 = .2678

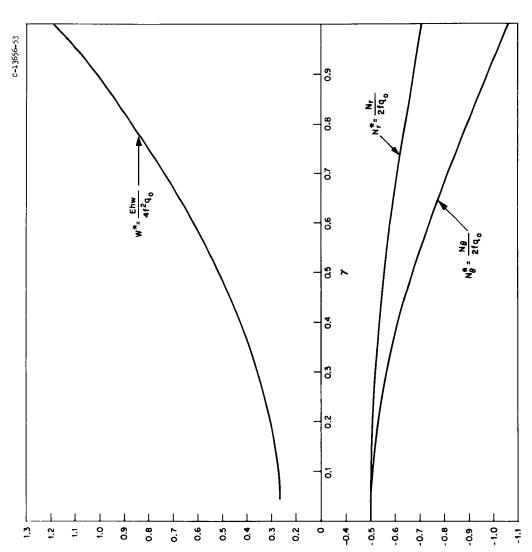
χ	* " Z	* ⁸ N	*0	**
0	500	009'-	0	1
.0446	500	5M	0004	.341
6990	501	503	9000'-	.344
.0892	504	505	0008	.347
. 1116	503	509	0009	.352
. 1339	504	513	00101	.357
.1562	506	518	00103	.364
.1785	507	523	00098	.371
.2008	509	529	0008	379
.2232	512	536	00066	.389
.2455	514	544	00037	399
.2678	517	552	000	410











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6.6 THE ASYMMETRIC MEMBRANE BEHAVIOR: GRAVITY LOADS

We now turn to the more difficult task of calculating the behavior of the paraboloidal shell of revolution under its own dead weight in the more general case when the axis of the paraboloid does not coincide with the axis of gravity (see figure 6.4.1). This means we must deal with the full set of membrane equations, 6.1.1 through 6.1.9, in which the loads are prescribed by equations 6.3.5, 6.3.6, and 6.3.7.

As a result of the closed character of the shell in the circumferential direction, the surface loading as well as the edge loading must be periodic functions of the angle θ with a period of 2π (see equations 6.3.5, 6.3.6, and 6.3.7). Therefore we have as eigenfunctions the set of trigonometric functions $\cos n\theta$ and $\sin n\theta$ for n=0, 1, 2, ---. It is well known, in the elementary theory of Fourier Series, that this set of eigenfunctions is complete. As N_r , N_θ , and $N_{r\theta}$ are sufficiently smooth, the usual expansion theorem applies. 20 , 21 Thus we can write

$$N_r(\xi_{\theta}) = a_0(\xi) + \sum_{n=1}^{\infty} \left[a_n(\xi) \sin n\theta + d_n(\xi) \cos n\theta \right], 6.6.1$$

$$N_{\theta}(x,\theta) = b_0(x) + \sum_{n=1}^{\infty} [b_n(x) \sin n\theta + e_n(x) \cos n\theta],$$
 6.6.2

$$N_{r\theta}(x,\theta) = f_0(x) + \sum_{n=1}^{\infty} \left[f_n(x) \sin n\theta + c_n(x) \cos n\theta \right].$$
 6.6.3

Substitution of these expressions into equations 6.1.4 through 6.1.6 and with the loads given by equations 6.3.5, 6.3.6, 6.3.7 yields the following equations (primes indicate differentiation with respect to ?):

$$xa_0' + a_0 - b_0 = 2f\rho_0 h \cos \psi \{x^2\},$$
 6.6.4

$$\frac{a_0}{1+(1)^2} + b_0 = 2f\rho_0 h \cos \psi , \qquad 6.6.5$$

$$8f_0' + 2f_0 = 0,$$
 6.6.6

$$a_1' + a_1 - c_1 \sqrt{1 + (8)^2} - b_1 = 2 + \rho_0 h \sin \psi \{ 8 \},$$
 6.6.7

$$\delta d_1' + d_1 + \sqrt{1 + (\delta)^2} \quad f_1 - e_1 = 0,$$
 6.6.8

$$\delta f_1' + f_1 - e_1 \sqrt{1 + (\gamma)^2} = 0,$$
 6.6.9

$$\delta C_1' + 2C_1 + b_1 \sqrt{1+(\gamma)^2} = -2f_{00}h \sin \psi \left\{ \delta \sqrt{1+(\delta)^2} \right\}, 6.6.10$$

$$\frac{a_1}{1+(1)^2} + b_1 = 2f\rho_0 h \sin \psi \{ \}, \qquad 6.6.11$$

$$\frac{d_1}{1+(1)^2} + e_1 = 0, 6.6.12$$

$$a_n' + a_n - n c_n \sqrt{1 + (a)^2} - b_n = 0,$$
 6.6.13

$$id_n' + d_n + n \sqrt{1 + (i)^2} f_n - e_n = 0$$

$$\delta f_n' + 2f_n - ne_n \sqrt{1 + (\gamma)^2} = 0$$

$$\chi_{C_n}' + 2C_n + nb_n \sqrt{1+(\chi)^2} = 0$$

$$\frac{a_n}{1+(\delta)^2}+b_n=0,$$

6.6.17

and

$$\frac{d_{n}}{1+(\gamma)^{2}}+d_{n}=0.$$

6.6.18

where n=2,3,4,---

With the usual boundary conditions imposed upon the stress resultants, we show next that equations 6.6.1, 6.6.2, and 6.6.3 must reduce to

$$N_r(Y, \theta) = a_0(Y) + a_1(Y) \sin \theta$$

6.6.19

$$N_{\bullet}(Y, \theta) = b_{o}(Y) + b_{1}(Y) \sin \theta,$$
 6.6.20

$$N_{re}(i, \theta) = C_1(i) \cos \theta.$$
 6.6.21

For a shell with a free edge at $V=\delta_1$, the homogeneous differential equation 6.6.6 for f_0 implies

$$[(\gamma)^2 f_0]' = 0$$
 6.6.22

or

$$f_0(Y) = \frac{A}{(Y)^2}$$
 6.6.23

Therefore, it follows that

$$f_0(r) \equiv 0.$$
 6.6.25

Next, equations 6.6.8, 6.6.9, and 6.6.12 can be uncoupled to yield the following relations:

$$f_1(y) = \frac{-1}{\sqrt{1+(y)^2}} \left\{ \delta d_1' + \left(1 + \frac{1}{1+(y)^2}\right) d_1 \right\},$$
 6.6.26

$$e_1(x) = \frac{-d_1}{1+(x)^2}$$
, 6.6.27

$$d_1'' + \left(\frac{5}{5} - \frac{27}{1 + (7)^2}\right) d_1' + \left(\frac{3}{(7)^2 \left[1 + (7)^2\right]^2}\right) d_1 = 0.$$
 6.6.28

Let the most general solution of the homogeneous equation 6.6.28 be

$$d_1(Y) = g_1h_1(Y) + g_2h_2(Y)$$
 6.6.29

where g_1 and g_2 are non-zero real constants and $h_1(Y)$ and $h_2(Y)$ are linearly independent solutions of equation 6.6.28.

From equation 6.6.26

$$f_{1}(x) = \frac{-1}{\sqrt{1+(x')^{2}}} \left\{ g_{1}[xh'_{1}(x) + (1+\frac{1}{1+(x')^{2}})h_{1}(x)] + g_{2}[xh'_{2}(x) + (1+\frac{1}{1+(x')^{2}})h_{2}(x)] \right\}.$$
6.6.30

From the boundary conditions, equations 6.2.5 and 6.2.6, we require

$$g_1 h_1 (x_1) + g_2 h_2 (x_1) = 0$$
 6.6.31

and

$$g_{1}\left[x_{1}h_{1}'(x_{1}) + \left(1 + \frac{1}{1 + (\delta_{1})^{2}}\right)h_{1}(x_{1})\right] + g_{2}\left[x_{1}h_{2}'(x_{1}) + \left(1 + \frac{1}{1 + (\delta_{1})^{2}}\right)h_{2}(\delta_{1})\right] = 0.$$
6.6.32

These latter two equations which are simultaneous homogeneous equations for the constants \mathbf{g}_1 and \mathbf{g}_2 possess non-trivial solutions only if the determinant of the coefficients of \mathbf{g}_1 and \mathbf{g}_2 vanishes.

If the determinant of the coefficients of g_1 and g_2 does

vanish, we have
$$h_1(x_1) \left[x_1 h_2'(x_1) + \left(1 + \frac{1}{1 + (x_1)^2} \right) h_2(x_1) \right]$$

$$-h_2(x_1) \left[x_1 h_1'(x_1) + \left(1 + \frac{1}{1 + (x_1)^2} \right) h_1(x_1) \right] = 0$$
6.6.33

or

$$h_1(x_1) h_2(x_1) - h_2(x_1) h_1(x_1) = 0.$$
 6.6.34

The left hand side of 6.6.34 is the Wronskian of the functions h_1 (%) and h_2 (%). The vanishing of the Wronskian at $Y = Y_1$ implies the linear dependence of h_1 (%) and h_2 (%) for all positive % and we have reached a contradiction. 18

Thus we can only have

$$g_1 = g_2 = 0$$
 6.6.35

or

$$d_1(Y) \equiv 0.$$
 6.6.36

Consequently (cf. equation 6.6.26 and 6.6.27)

$$f_1(x) \equiv 0, \qquad 6.6.37$$

$$e_1(x) \equiv 0.$$
 6.6.38

By repetitions of the above argument, we can also show

that

$$f_{n}(Y) \equiv 0, \qquad 6.6.39$$

$$f_{n}(Y) \equiv 0, \qquad 6.6.140$$
and
$$e_{n}(Y) \equiv 0 \qquad 6.6.141$$

$$for all n and
$$a_{n}(Y) \equiv 0, \qquad 6.6.142$$

$$b_{n}(Y) \equiv 0, \qquad 6.6.143$$
and
$$c_{n}(Y) \equiv 0 \qquad 6.6.141$$$$

for $n \ge 2$.

In short we have the very desirable results that

$$N_r(x, \theta) = a_0(x) + a_1(x) \sin \theta,$$
 6.6.45

$$N_{\theta}(x,\theta) = b_{0}(x) + b_{1}(x) \sin \theta, \qquad 6.6.46$$

and $N_{r\theta} = C_1(t) \cos \theta$. 6.6.47

If the shell is closed at the apex, the usual condition imposed upon the stress resultants is that they be finite. 10,11 The

application of this condition to equation 6.6.23 results in

$$f_0(x) \equiv 0.$$
 6.6.48

Next we observe that equation 6.6.28 has a regular singularity at J=0. Therefore, in the neighborhood of the apex, the most general solution of 6.6.28 takes the form

$$d_{1}(i) = C_{1} \sum_{n=0}^{\infty} S_{n} i^{n+m_{1}} + C_{2} \left\{ \ln (i) \sum_{n=0}^{\infty} S_{n} i^{n+m_{1}} + \sum_{n=0}^{\infty} t_{n} i^{n+m_{2}} \right\}$$

$$6.6.49$$

where C_1 and C_2 are constants of integration to be determined by the known side constraints, s_n and t_n are constants fixed by the relevant recurrence relations, m_1 and m_2 ($m_1 \ge m_2$) are roots of the indicial equation

$$m^2 + 4m + 3 = 0,$$
 6.6.50

or
$$m_1 = -1$$
, 6.6.51

and
$$m_2 = -3$$
. 6.6.52

To ensure finiteness at the apex, we must have

$$C_1 = C_2 = 0$$
 6.6.53

that

and

and

$$d_{n}(Y) \equiv 0, \qquad 6.6.39$$

$$f_{n}(Y) \equiv 0, \qquad 6.6.10$$
and
$$e_{n}(Y) \equiv 0 \qquad 6.6.11$$
for all n and
$$a_{n}(Y) \equiv 0, \qquad 6.6.12$$

$$b_{n}(Y) \equiv 0, \qquad 6.6.13$$

 $c_n(x) \equiv 0$

for $n \ge 2$.

In short we have the very desirable results that

6.6.44

$$N_{\Gamma}(Y,\theta) = a_{O}(Y) + a_{1}(Y) \sin \theta, \qquad 6.6.45$$

$$N_{\theta}(Y,\theta) = b_{O}(Y) + b_{1}(Y) \sin \theta, \qquad 6.6.46$$
and
$$N_{\Gamma\theta} = c_{1}(Y) \cos \theta. \qquad 6.6.47$$

If the shell is closed at the apex, the usual condition imposed upon the stress resultants is that they be finite. 10,11 The

and consequently

$$d_1(\delta) = 0, \qquad 6.6.54$$

$$e_1(1) = 0,$$
 6.6.55

and
$$f_1(x) \equiv 0$$
. 6.6.56

Repetitions of the above argument will lead to the conclusion that $a_n(t)$, $b_n(t)$, $c_n(t)$, $d_n(t)$, $e_n(t)$, and $f_n(t)$ must also vanish identically for $n \ge 2$. We have again arrived at the very desirable results that

$$N_r(x, \theta) = a_0(x) + a_1(x) \sin \theta,$$
 6.6.57

$$N_{\theta}(x,\theta) = b_{\theta}(x) + b_{1}(x) \sin \theta,$$
 6.6.58

and

$$N_{r\theta}(x,\theta) = c_1(x) \cos \theta. \qquad 6.6.59$$

Thus the problem of the membrane stress resultants is reduced to the ascertainment of solutions to the system of differential

equations (6.6.4), (6.6.5), (6.6.7), (6.6.10) and (6.6.11). Equations 6.6.4 and 6.6.5 completely determine $a_0(Y)$ and $b_0(Y)$ up to a constant of integration. It should be observed that these are the portions of the N_r and N_0 induced by the axi-symmetric component of the load. This axi-symmetric portion has the same dependence on Y except for a factor $\cos \Psi$ as was found earlier for the completely axi-symmetric behavior (cf. equations 6.4.14 and 6.4.15).

Thus

$$a_0(y) = \frac{2f\rho_0 h \cos \psi}{3} \frac{\sqrt{1+(Y)^2}}{(Y)^2} \left\{ \left[1+(Y)^2 \right]^{3/2} + C_1 \right\}$$
 6.6.60

and

$$b_0(1) = 2f\rho_0h \cos \psi \left\{ 1 - \frac{1}{3(1)^2 \sqrt{1+(1)^2}} \left[\left[1 + (1)^2 \right]^{3/2} + C_1 \right] \right\} 6.6.61$$

where C_1 is a constant of integration to be determined by the side condition of the shell.

The remaining three equations, 6.6.7, 6.6.10, 6.6.11, can be reduced to the following three uncoupled equations

$$a_1'' + \left(\frac{5}{8} - \frac{27}{1 + (8)^2}\right) a_1' + \frac{3}{(7)^2 [1 + (8)^2]^2} a_1 = -4f_{\rho_0}h \sin \psi \left\{\frac{1 + (8)^2}{8}\right\},$$

$$6.6.62$$

$$b_1(x) = \frac{-a_1(x)}{1+(x)^2} + 2f_{\rho_0}h \sin \psi \left\{ x \right\},$$
6.6.63

$$c_{1}(x) = \frac{1}{\sqrt{1+(x)^{2}}} \left\{ x a_{1}'(x) + \left(1 + \frac{1}{1+(x)^{2}}\right) a_{1}(x) \right\}$$
6.6.64

and we need only to solve the differential equation 6.6.62 to complete the analysis of membrane stresses.

Observe that (6.6.62) can be rewritten as

$$\frac{\sqrt{1+(\gamma)^2}}{(\gamma)^2} \frac{d}{d\gamma} \left\{ \frac{1}{\gamma} \frac{d}{d\gamma} \left(\frac{(\gamma)^3 a_1}{\sqrt{1+(\gamma)^2}} \right) \right\} = -4f\rho_0 h \sin \psi \left\{ \frac{1+(\gamma)^2}{\gamma} \right\}$$

or

$$\frac{d}{d_{\delta}} \left\{ \frac{1}{\delta} \frac{d}{d_{\gamma}} \left(\frac{(\delta)^{3} a_{1}}{\sqrt{1 + (\delta)^{2}}} \right) \right\} = -4f_{\rho_{0}h} \sin \psi \left\{ \delta \sqrt{1 + (\delta)^{2}} \right\}.$$
6.6.65

Thus
$$\frac{d}{dy} \left(\frac{(y)^3 a_1}{\sqrt{1+(y)^2}} \right) = -4 f \rho_0 h \sin \psi \left\{ \frac{(1+(y)^2)^{3/2}}{3} + C_3 \right\} \left\{ y \right\} .$$
6.6.66

$$\frac{(i)^3 a_1}{\sqrt{1+(i)^2}} = -4f \rho_0 h \sin \psi \left\{ c_4 + \frac{c_3(i)^2}{2} + \frac{(1+(i)^2)^{5/2}}{15} \right\},$$
6.6.67

and
$$a_1(\vec{x}) = -4f\rho_0h \sin \phi \left\{ \frac{C_4\sqrt{1+(\vec{x})^2}}{(\vec{x})^3} + \frac{C_3\sqrt{1+(\vec{x})^2}}{2\vec{x}} + \frac{(1+(\vec{x})^2)^3}{15(\vec{x})^3} \right\}_{6.6.68}$$

where C_3 and C_h are commutants of integration.

 $b_1(1)$ and $c_1(1)$ can then be computed with the help of equations (6.6.63) and (6.6.64) to be

$$b_1(t) = 2f_{p_0}h \sin \psi \left\{ \delta + \frac{2}{(1)^3\sqrt{1+(1)^2}} \left[C_4 + \frac{C_3(t)^2}{2} + \frac{(1+(t)^2)^{5/2}}{15} \right] \right\},$$
6.6.69

$$C_1(1) = 4f\rho_0 h \sin \psi \left\{ \frac{\sqrt{1+(1)^2}}{15(1)^3} \left(1 - 4(1)^2 \right) - \frac{C_3}{21} + \frac{C_4}{(1)^3} \right\}.$$
6.6.70

Finally, from (6.6.19), (6.6.20), and (6.6.21), we have

$$N_{\Gamma}(\delta,\theta) = \frac{2f\rho_0 h \cos \psi}{3} \frac{\sqrt{1+(\xi)^2}}{(\xi)^2} \left\{ \left(1+(\xi)^2\right)^{3/2} + C_1 \right\} -4f\rho_0 h \sin \psi \left\{ \frac{C_3\sqrt{1+(\xi)^2}}{2\chi} + \frac{C_4\sqrt{1+(\xi)^2}}{(\xi)^3} + \frac{\left(1+(\chi)^2\right)^3}{15(\xi)^3} \right\} \sin \theta,$$
6.6.71

$$N_{\theta}(\delta,\theta) = 2f\rho_{0}h \cos \psi \left\{ 1 - \frac{1}{3(\delta)^{2}\sqrt{1+|\gamma|^{2}}} \left[(1+(\delta)^{2})^{3/2} + C_{1} \right] \right\}$$

$$+ 2f\rho_{0}h \sin \psi \left\{ \delta + \frac{2}{(\delta)^{3}\sqrt{1+|\gamma|^{2}}} \left[C_{4} + \frac{C_{3}(\delta)^{2}}{2} + \frac{(1+(\delta)^{2})^{5/2}}{15} \right] \right\} \sin \theta,$$

$$6.6.72$$

and

$$N_{\Gamma\theta}(x,\theta) = 4f_{\rho_0}h \sin \psi \left\{ \frac{(1+(x)^2)^{3/2}}{15(x)^3} \left(1-4(x)^2\right) - \frac{C_3}{2x} + \frac{C_4}{(x)^3} \right\} \cos \theta.$$
6.6.73

To determine the displacements, the stress-strain relations (cf. equations 6.1.7, 6.1.8, 6.1.9) and the strain-displacement relations (cf. equations 6.1.1, 6.1.2, 6.1.3) are combined to yield the following set of partial differential equations for the displacements u_r^0 , u_θ^0 and w:

$$\frac{\partial u_r^0}{\partial r} - \frac{w}{1 + (\delta)^2} = \frac{2f\sqrt{1 + (\delta)^2}}{Eh} \left[N_r - \nu N_\theta \right] = 2f\sqrt{1 + (\delta)^2} \quad \epsilon_r^0,$$

$$\sqrt{1+(1)^2} \frac{\partial u_{\theta}^{\circ}}{\partial \theta} + u_{r}^{\circ} - \delta w = \frac{2f \sqrt{1+(1)^2}}{Eh} \left[N_{\theta} - \nu N_{r} \right] = 2f \sqrt{1+(1)^2} \in_{\theta}^{\circ},$$
6.6.75

$$y \frac{\partial u_{\theta}^{\circ}}{\partial x} - u_{\theta}^{\circ} + \sqrt{1 + (\delta)^{2}} \frac{\partial u_{r}^{\circ}}{\partial \theta} = \frac{2fy\sqrt{1 + (\delta)^{2}}}{Eh} \left[2(1 + \nu)N_{r\theta} \right]$$

$$= 2fy\sqrt{1 + (\delta)^{2}} \left[2\epsilon_{r\theta}^{\circ} \right].$$
6.6.76

If the shell is closed at the apex or has a free edge, the form of the solutions for the stress-resultants (cf. equations 6.6.71, 6.6.72, 6.6.73) and boundary conditions on U_r^o and U_θ^o (cf. equations 6.2.7 and 6.2.8) suggest that the solutions for the displacements should take on the forms,

$$u_r^o(x,\theta) = m_o(x) + m_s(x) \sin \theta,$$
 6.6.77

$$u_{\theta}^{o}(\delta,\theta) = n_{1}(\delta) \cos \theta,$$
 6.6.78

$$W(x, \theta) = p_0(x) + p_1(x) \sin \theta$$
. 6.6.79

The proof goes through as that for the stress resultants. (cf. pp 33-37)

The higher harmonic terms which were omitted from equations (6.6.77), (6.6.78) and (6.6.79) correspond to displacements without straining the middle surface of the shell and are often referred to as the inextensional deformations. Thus we may conclude that shells with the given displacement conditions exhibit no inextensional deformations (cf. page 7).

Equations (6.6.77) through (6.6.79) hold also for the case where the shell is fixed tangentially at both edges. Here the inextensional

deformations must be of the form

$$u_{rn}(\vec{s},\theta) = m_{n}(\vec{s}) \left\{ \begin{array}{l} (\sin n\theta) \\ (\cos n\theta) \end{array} \right\}$$

$$= \left\{ \begin{array}{l} c_{1}f_{1}(\vec{s},n) + c_{2}f_{2}(\vec{s},n) + c_{3}f_{3}(\vec{s},n) + c_{4}f_{4}(\vec{s},n) \right\} \left\{ \begin{array}{l} (\cos n\theta) \\ (\vec{s},\theta) \end{array} \right\}$$

$$= \left\{ \begin{array}{l} (\cos n\theta) \\ (\vec{s},\theta) \end{array} \right\}$$

$$= \left\{ \begin{array}{l} (c_{1}g_{1}(\vec{s},n) + c_{2}g_{2}(\vec{s},n) + c_{3}g_{3}(\vec{s},n) + c_{4}g_{4}(\vec{s},n) \right\} \left\{ \begin{array}{l} (\cos n\theta) \\ (\sin n\theta) \end{array} \right\}$$

The constants of integration C_1 , C_2 , C_3 and C_4 are to be determined by the displacement boundary conditions (6.2.1) through (6.2.4). Thus, we have

$$C_{1}f_{1}(x_{1},n)+C_{2}f_{2}(x_{1},n)+C_{3}f_{3}(x_{1},n)+C_{4}f_{4}(x_{1},n)=0,$$

$$C_{1}f_{1}(x_{2},n)+C_{2}f_{2}(x_{2},n)+C_{3}f_{3}(x_{2},n)+C_{4}f_{4}(x_{2},n)=0,$$

$$C_{1}g_{1}(x_{1},n)+C_{2}g_{2}(x_{1},n)+C_{3}g_{3}(x_{1},n)+C_{4}g_{4}(x_{1},n)=0,$$

$$C_{1}g_{1}(x_{2},n)+C_{2}g_{2}(x_{2},n)+C_{3}g_{3}(x_{2},n)+C_{4}g_{4}(x_{2},n)=0.$$

Since $M_n(Y)$ and $N_n(Y)$ are linearly independent, the set of linear algebraic equations for C_1 , C_2 , C_3 and C_4 admits no non-trivial solutions.

Substitution of the expressions (6.6.77) through (6.6.79) into the system of partial differential equations 6.6.74, 6.6.75 and 6.6.76 yields the following set of five ordinary differential equations for the five unknown functions m_0 , m_1 , n_1 , n_2 , and n_3 (primes indicate

differentiation with respect to 7):

$$m_0' - \frac{p_0}{1 + (1)^2} = 2f \sqrt{1 + (1)^2} \in r^*,$$
 6.6.80

$$m_0 - yp_0 = 2f \sqrt[3]{1 + (\sqrt[3]{2})^2} \in {}_{\Theta}^{*},$$
 6.6.81

$$m_1' - \frac{P_1}{1 + (1)^2} = 2f \sqrt{1 + (1)^2} \quad \epsilon_r^{(1)},$$
 6.6.82

$$-\sqrt{1+(\gamma)^2} \quad n_1 + m_1 - \gamma p_1 = 2f \gamma \sqrt{1+(\gamma)^2} \quad \epsilon_{\theta}^{(1)}, \qquad 6.6.83$$

$$\forall n_1' - n_1 + \sqrt{1 + (\gamma)^2} \quad m_1 = 4f \gamma \sqrt{1 + (\gamma)^2} \in_{r\theta}^{(1)}$$
6.6.84

where for convenience we have separated the middle surface strains into components, defined as follows:

$$\epsilon_{\Gamma}^{0}(\xi,\theta) = \epsilon_{\Gamma}^{*}(\xi) + \epsilon_{\Gamma}^{(1)}(\xi) \sin \theta$$
, 6.6.85

$$\epsilon_{\theta}^{\circ}(\mathbf{z},\theta) = \epsilon_{\theta}^{\dagger}(\mathbf{z}) + \epsilon_{\theta}^{(1)}(\mathbf{z}) \sin \theta$$
,

6.6.86

$$\epsilon_{r\theta}^{\circ}(\mathbf{r},\theta) = \epsilon_{r\theta}^{(1)} \cos \theta.$$
6.6.87

It was noted, in a previous paragraph in this section, that the solutions for the force-resultants N_r and N_θ contained an axi-symmetric portion identical in form (except for a multiplicative factor of $\cos \psi$) to the solutions determined in section 6.4. Thus the results of section 6.4, can be used to determine the solutions for $m_\theta(r)$ and $p_0(r)$, the axi-symmetric portions of the solutions for the displacements u_r^0 and w. There results (compare equations 6.4.20, 6.4.18 with equations 6.6.81 and 6.6.82):

$$m_{0}(y) = \frac{4f^{2}\rho_{0}\cos\psi}{3E\sqrt{1+(1)^{2}}} \left\{ (1+\nu)\left(Y \ln x - \frac{1}{y}\right) + \left(\frac{3-\nu}{2}\right)(y)^{3} + \frac{1}{4}(y)^{5} + C_{1}\left[-(1+\nu)Y \ln\left(\frac{1+\sqrt{1+(Y)^{2}}}{y}\right) + \sqrt{1+(y)^{2}}\left(Y - \frac{1+\nu}{y}\right) \right] + C_{2}Y \right\},$$

$$6.6.88$$

$$p_{0}(\delta) = \frac{4f^{2}\rho_{0}\cos\psi}{3E\sqrt{1+(1)^{2}}} \left\{ (1+\nu)\left(\ln\delta - \frac{1}{(\delta)^{2}}\right) + \left(\frac{3-\nu}{2}\right)(\delta)^{2} + \frac{1}{4}(\delta)^{5} + \left(1+(\delta)^{2}\right)\left[2(\nu-1) + \frac{1+\nu}{(\delta)^{2}} + \nu(\delta)^{2}\right] + c_{1}\left[-(1+\nu)\ln\left(\frac{1+\sqrt{1+(\delta)^{2}}}{\gamma}\right) + \sqrt{1+(\delta)^{2}}(1+\nu)\right] + c_{2}\right\}$$

$$= \frac{4f^{2}\rho_{0}\cos\psi}{3E\sqrt{1+(1)^{2}}} \left\{ (1+\nu)\left(\ln\delta - \frac{1}{(\delta)^{2}}\right) + \left(\frac{3-\nu}{2}\right)(\delta)^{2} + \frac{1}{4}(\delta)^{5} + \left(\frac{1+\nu}{2}\right)^{2}\right\}$$

$$= \frac{4f^{2}\rho_{0}\cos\psi}{3E\sqrt{1+(1)^{2}}} \left\{ (1+\nu)\left(\ln\delta - \frac{1}{(\delta)^{2}}\right) + \left(\frac{3-\nu}{2}\right)(\delta)^{2} + \frac{1}{4}(\delta)^{5} + \left(\frac{1+\nu}{2}\right)(\delta)^{2} + \frac{1}{4}(\delta)^{5} + \left(\frac{1+\nu}{2}\right)(\delta)^{2} + \frac{1}{4}(\delta)^{2} + \frac{$$

where C_1 is the constant of integration arisen from the integration of force equilibrium equation and C_2 is an additional constant from the integration of equation 6.6.80.

The three remaining equations, 6.6.82, 6.6.83, 6.6.84, which determine the asymmetric component of the displacements can be rearranged to yield

$$\left(\frac{\mathsf{n}_{1}'}{\mathsf{y}}\right)' = \frac{2\mathsf{f}}{\mathsf{y}} \left\{ \left[\sqrt{1+(\mathsf{y})^{2}} \, 2 \, \epsilon_{\mathsf{r}\,\theta}^{(1)} \right]' + \frac{\epsilon_{\mathsf{\theta}}^{(1)} - \epsilon_{\mathsf{r}}^{(1)}}{\mathsf{y}} - \mathsf{y} \, \epsilon_{\mathsf{r}}^{(1)} \right\},$$

$$6.6.90$$

$$m_1 = Af X \in_{r\theta}^{(1)} - \frac{Y}{\sqrt{1+(Y)^2}} \quad n_1' + \frac{n_1}{\sqrt{1+(Y)^2}},$$
6.6.91

$$p_1 = -2f \left(1+(1)^2\right)^{3/2} \mathcal{E}_{\Gamma}^{(1)} + \left(1+(1)^2\right) m_1'.$$
6.6.92

We can obtain the solution for n_i by a simple, though

tedious, double quadrature

$$n_{1}(x) = 2f \left\{ \int_{0}^{x} \left[x \int_{0}^{x} \left(\frac{\left[\sqrt{1 + (x)^{2}} 2 \varepsilon_{r \theta}^{(1)}\right]'}{x} + \frac{\varepsilon_{\theta}^{(1)} - \varepsilon_{r}^{(1)}}{(x)^{2}} - \varepsilon_{r}^{(1)} \right) \partial x \right] dx$$

$$+ \frac{c_{5}(x)^{2}}{2} + c_{6} \right\}$$

$$6.6.93$$

or
$$n_1(x) = \frac{4f\rho_0 \sin \Psi}{E} \left\{ C_6 + \frac{C_5(x)^2}{2} + C_4f_2(x) + C_3f_1(x) + f_3(x) \right\}$$

6.6.94

where

$$f_{1}(x) = \frac{\left(1+(t)^{2}\right)^{3/2}}{6} - \left(1+\nu\right) \left[\sqrt{1+(t)^{2}} + \ell_{n}\left(\sqrt{1+(t)^{2}} - 1\right) - \ell_{n}(x)\right],$$

$$6.6.95$$

$$f_{2}(x) = \frac{\left(1-\nu\right)}{4} \left(x\right)^{2} \ell_{n} \left[\frac{\sqrt{1+(t)^{2}} - 1}{x}\right] + \frac{\left(1-\nu\right)}{4} \left[1+\sqrt{1+(t)^{2}}\right] - \frac{\left(1+\nu\right)}{2} \frac{\left[1+(t)^{2}\right]^{3/2}}{(t)^{2}},$$

6.6.96

$$f_3(x) = \frac{(x)^6}{360} - \frac{(40 + 29 \nu)}{240} (x)^4 - \frac{(1 + \nu)}{30(x)^2} - \frac{(1 - \nu)(x)^2}{120} + I_n(x) \left[\frac{(1 - \nu)}{60} (x)^2 - \frac{(1 + \nu)}{30} \right],$$

$$6.6.97$$

 c_{5} and c_{6} are constants of integration.

By equations 6.6.91 and 6.6.83, we have also

$$m_{1}(\delta) = \frac{\theta f^{2} \rho \sin \psi}{E} \left\{ C_{3} f_{4}(\delta) + C_{4} f_{5}(\delta) - \frac{C_{5}(\delta)^{2}}{2\sqrt{1+(\delta)^{2}}} + \frac{C_{6}}{\sqrt{1+(\delta)^{2}}} + f_{6}(\delta) \right\}$$
6.6.98

and

$$p_{1}(t) = \frac{8t^{2}\rho \sin \psi}{E} \left\{ C_{3}f_{7}(\delta) + C_{4}f_{8}(\delta) + C_{5}f_{9}(\delta) + C_{6}f_{10}(t) + f_{11}(\delta) \right\}$$
6.6.99

where

$$f_4(x) = \frac{f_1(x)}{\sqrt{1+(x)}^2} - \frac{(x)^2}{2} ,$$
6.6.100

$$f_{5}(x) = \frac{f_{2}(\delta)}{\sqrt{1+(x)^{2}}} + \frac{(1+\nu)}{(x)^{2}} \left(\frac{(x)^{2}}{2} + 1\right) - \frac{(1-\nu)(x)^{2}}{2\sqrt{1+(x)^{2}}} \left[I_{1}\left(\frac{\sqrt{1+(\delta)^{2}} - 1}{x}\right) \right]$$
6.6.101

$$f_{\zeta}(\xi) = \frac{f_{3}(\delta)}{\sqrt{1+(\xi)^{2}}} - \frac{\xi}{\sqrt{1+(\xi)^{2}}} \left[\frac{(1-\nu)}{30} \xi \ln(\xi) + \frac{(1+\nu)}{15(\xi)^{3}} - \frac{(1+\nu)}{3\delta} - \frac{(40+29\nu)}{60} \right] + \frac{(1)^{5}}{60} + \frac{2(1+\nu)\left[1-4(\xi)^{2}\right]\left[1+(\xi)^{2}\right]^{3/2}}{15(\xi)^{2}}$$

$$= \frac{(1)^{5}}{60} + \frac{2(1+\nu)\left[1-4(\xi)^{2}\right]\left[1+(\xi)^{2}\right]^{3/2}}{15(\xi)^{2}}$$

$$= \frac{6.6.102}{60}$$

$$f_7(x) = \frac{1}{x} \left[f_4(x) - \sqrt{1 + (x)^2} f_1(x) - \frac{(1+\nu)}{2} - \frac{\nu(x)^2}{2} \right],$$
6.6.103

$$f_8(8) = \frac{1}{8} \left[f_5(8) - \sqrt{1 + (8)^2} f_2(8) - \frac{1}{(8)^2} (1 + \nu) - \nu \right],$$
6.6.104

$$f_9(x) = \frac{-\frac{1}{2}(\delta)}{\sqrt{1+(x)^2}} \left[2+(x)^2\right],$$
6.6.105

$$f_{10}(3) = \frac{-3}{\sqrt{1+(3)^2}}$$
,
6.6.106

$$f_{11}(x) = \frac{1}{\delta} \left[f_{\delta}(x) - \sqrt{1 + (x)^2} + \frac{1}{3} (x) - \frac{\sqrt{1 + (x)^2}}{(x)^2} \left\{ \frac{(x)^4}{2} + \frac{[1 + (x)^2]^2}{15} + \frac{\nu[1 + (x)^2]^3}{15} \right\} \right].$$
6.6.107

Several different combinations of boundary conditions are considered and numerical calculations have been made. It is evident that a complete description of the deformed shell for each orientation (i.e., each value of ψ) would require an inordinate number of curves and tables. In lieu of presenting such a mass of numerical data only the anti-symmetric part of the solution has been presented ($\psi = \frac{\pi}{2}$). The behavior of the shell for other values of ψ can be obtained by appropriate combinations of the results for $\psi = 0$ and $\psi = \frac{\pi}{2}$. The solutions for the force resultants and displacements can be cast into the forms (see equations 6.6.19, 6.6.20, 6.6.21, 6.6.77, 6.6.78, 6.6.79,

6.6.69, 6.6.70, 6.6.71, 6.6.72, 6.6.73, 6.6.94, 6.6.98, and 6.6.99),

$$N_r^* = \frac{N_r}{2f_{lh}} = a_c^* \cos \Psi + a_1^* \sin \theta \sin \Psi \qquad 6.6.108$$

$$N_{\theta}^{*} = \frac{N_{\theta}}{2f\rho_{0}h} = b_{0}^{*} \cos \Psi + b_{1}^{*} \sin \theta \sin \Psi$$
 6.6.109

$$N_{r\theta}^* = c_1^* \cos \theta \sin \Psi$$
 6.6.110

$$u_n^* = \frac{Eu_n^0}{4f_{\rho_0}^2} = m_0^* \cos \Psi + m_1^* \sin \Theta \sin \Psi$$
 6.6.111

$$u_{\theta}^{*} = \frac{E u_{\theta}^{\circ}}{4f^{2}\rho_{0}} = n_{1}^{*} \cos \theta \sin \Psi \qquad 6.6.112$$

$$W^* = \frac{EW}{4f^2\rho_0} = \rho_c^* \cos \Psi + \rho_1^* \sin \theta \sin \Psi \qquad 6.6.113$$
The a_0^* , b_c^* , m_0^* , ρ_0^* are the non-dimensionalized

symmetric parts of the solution.

$$a_o^* = \frac{a_o}{2f\rho_o h \cos \psi}$$
 6.6.114

$$b_o^* = \frac{b_o}{2f\rho_o h \cos \Psi}$$
 6.6.115

$$m_0^* = \frac{Em_0}{4f^2\rho_0\cos\psi}$$
 6.6.116

$$\rho_o^* = \frac{E \rho_o}{4f^2 \rho_o \cos \Psi}$$
 6.6.117

1

These have already been plotted in the previous sections of this chapters for the case of ψ = 0. The other starred quantities are the asymmetric part of the solution and are defined as follows:

$$a_1^* = \frac{E a_1}{2f \rho_0 h \sin \Psi}$$
 6.6.118

$$b_{1}^{*} = \frac{E b_{1}}{2 f \rho_{c} h \sin \Psi}$$

$$c_{1}^{*} = \frac{E c_{1}}{2 f \rho_{c} h \cos \Psi}$$

$$m_{1}^{*} = \frac{E m_{1}}{4 f^{2} \rho_{c} \sin \Psi}$$

$$6.6.121$$

$$n_{1}^{*} = \frac{E n_{1}}{4 f^{2} \rho_{c} \sin \Psi}$$

$$6.6.122$$

$$p_{1}^{*} = \frac{E p_{1}}{4 f^{2} \rho_{c} \sin \Psi}$$

$$6.6.123$$

The results which are presented in the following sections are the above six functions for the case of $\psi = \frac{\pi}{2}$. Thus, the results represent the completely anti-symmetric behavior $N_r^*, N_\theta^*, N_{r\theta}^*, u_r^*, u_\theta^*$, and w^* . Additionally, it is seen that a_1^*, b_1^*, m_1^* and p_1^* represent the stress resultants and displacements along the bottom vertical radius $(\theta = \frac{\pi}{2})$ whereas c_1^* and c_1^* represent c_1^* and c_2^* along the horizontal radius c_1^* and c_2^* and c_3^* are present c_1^* and c_2^* along the horizontal radius c_2^* and c_3^* are present c_4^* and c_3^* along the horizontal radius c_3^* and c_4^* are present c_4^* and c_5^* and c_6^* along the horizontal radius c_5^* and c_6^* are obtained by multiplying the given values by c_5^* or c_6^* and c_6^* whichever is appropriate.

6.6.1 BOTH EDGES RESTRAINED IN THE TANGENTIAL DIRECTIONS

The condition of vanishing tangential displacements at the edges requires that

$$C_3f_1(s_1)+C_4f_2(s_1)+c_5\frac{(s_1)^2}{2}+c_6=-f_3(s_1)$$
,
6.6.1.1

$$c_3f_1(Y_2) + c_4f_4(Y_2) + \frac{c_5(Y_2)^2}{2} + c_4 = -f_3(Y_2)$$
6.6.1.2

$$c_3 f_4(Y_1) + c_4 f_5(Y_1) - \frac{c_5(Y_2)^2}{2\sqrt{1+(Y_1)^2}} + \frac{c_6}{\sqrt{1+(Y_1)^2}} = -f_6(Y_1)$$
 6.6.1.3

$$c_3 f_4(\zeta_2) + c_4 f_5(\zeta_2) - \frac{c_5(\zeta_2)^2}{2\sqrt{1+(\zeta_2)^2}} + \frac{c_6}{\sqrt{1+(\zeta_2)^2}} = -f_6(\zeta_2)$$
6.6.1.4

All f_i (%) have been defined previously. The system of equations (6.6.1.1) through (6.6.1.4) can be solved simultaneously for C_3 , C_4 C_5 and C_6 .

Six different cases have been analyzed and the results are presented in curve form as well as in tabular form. It should be observed that tangential displacements, as represented by M_1^* and n_1^* , become of the same order of magnitude as the normal displacement (represented by φ_1^*) over a portion of the shell. This is in contrast to the symmetric behavior in which the tangential displacement is generally an order of magnitude smaller than the normal displacement (see section 6.4). Also, the largest value of φ_1^* (which is W^* at $\theta = \frac{\pi}{2}$ and $\mathsf{W}^* = \frac{\pi}{2}$) is larger than W^* for the symmetric case (compare case 6.6.1.2 with 6.4.1.2 and case 6.6.1.4 with 6.4.1.4). This is a bit surprising until it is remembered that the value of W^* at the boundaries is determined primarily by the need to accommodate the membrane boundary conditions (see discussions in

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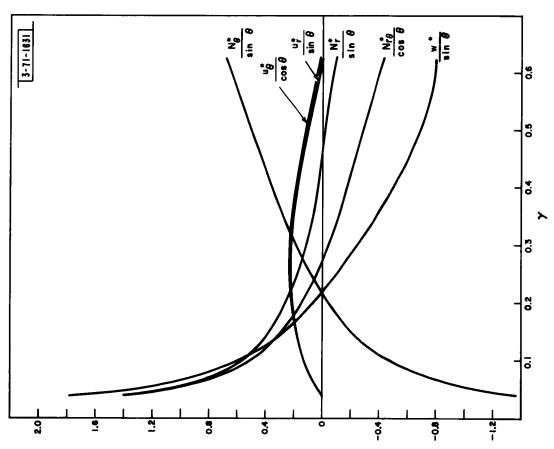
sections 6.4.1, 6.4.2, and 6.4.3).

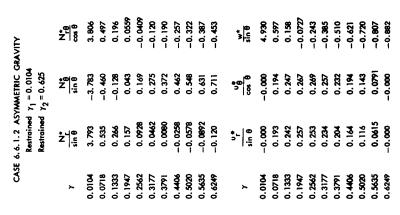
The list of configurations which have been analyzed are presented in Table 6.6.1.

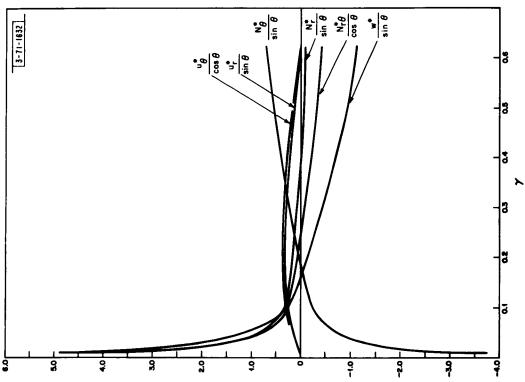
Table 6.6.1

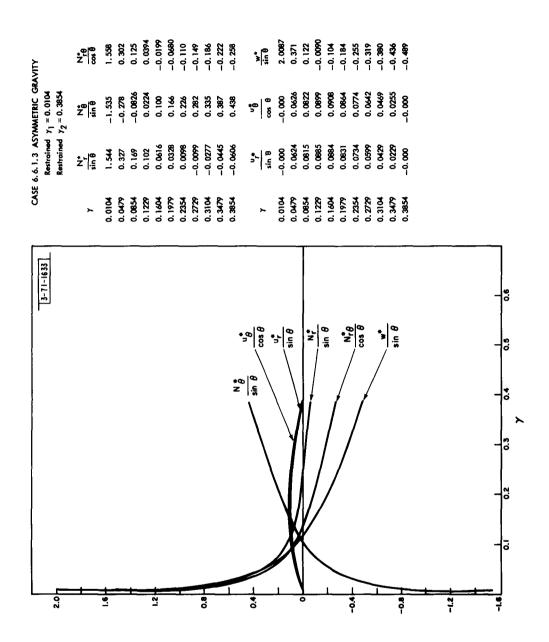
Case	Restrained at δ_1	Restrained at
6.6.1.1	.0417	.6250
6.6.1.2	.0104	.6250
6.6.1.3	.0104	. 3854
6.6.1.4	. 3854	.6250
6.6.1.5	.0416	• 799 0
6.6.1.6	.0416	1.0243

AVITY	χ 00 • 10 • 10 • 10 • 10 • 10 • 10 • 10 •	1.390	0.515	0.253	0.107	0.0042	-0.0803	-0.155	-0.224	-0.291	-0.356	-0.422		** Buis	1. 186	909 .0	0.224	-0.0086	-0.184	-0.331	-0.458	-0.571	-0.672	0.760	-0.838
YMMETRIC GR $\gamma_1 = 0.0417$ $\gamma_2 = 0.625$	X is	-1.359	-0.462	-0.170	0.0036	0.135	0.245	0.344	0.436	0.522	0.605	, 0.685	,	60 80 0 80	-0.000	0.127	0.184	0.212	0. 221	0.216	0.198	0.168	0.125	0.0697	0.00
CASE 6. 6. 1. 1 ASYMMETRIC GRAVITY Restrained $\gamma_1=0.0417$ Restrained $\gamma_2=0.625$	χ ς Θ	1.403	0.567	0.337	0. 223	0.150	0.0972	0.0541	0.0164	-0.0181	-0.0514	-0.0843	,	sin B	-0.000	0.125	0.178	0. 201	0. 205	0.194	0.172	0.140	0.100	0.0535	0.00
CASE	>	0.0416	0.0999	0.1583	0.2166	0. 2749	0.3333	0.3916	0.4499	0.5083	0.5666	0.6249		ب	0.0416	0.0999	0.1583	0.2166	0. 2749	0.3333	0.3916	0.4499	0.5083	0.5666	0.6249



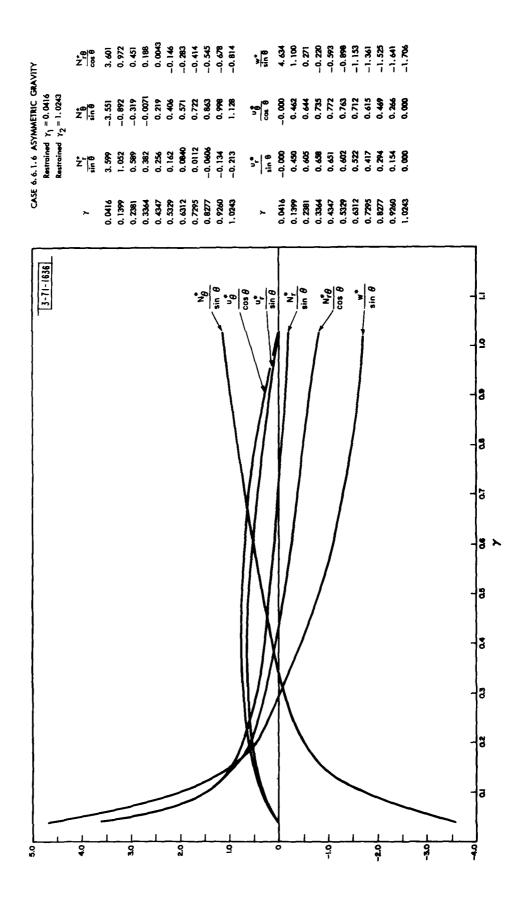






						3-71-1634	CASE	CASE 6.6.1.4 ASYMMETRIC GRAVITY Restrained $\gamma_1 = 0.3854$	WMETRIC GRA	VIIY
9	1					* 0		Restrained $\gamma_2 = 0.625$	r ₂ = 0.625	
•						eis Vis	>	Žμ	X vin Quis	χ 8 8
ð	1			/	\	\	0,3854	0.403	0.0339	0.241
				/	\		0.4093	0.373	0.0891	0.186
•	i				X		0. 4333	0.345	0.142	o. 134
-	,			/	/ \	•	0.4572	0.319	0.192	0.0868
25	1					<u> </u>	0.4812	0. 294	0.241	0.0415
				X		u sin 8	0.5052	0. 270	0.289	-0.0014
_				\	\ <u>\</u>	μn θ 603	0.5291	0.248	0.335	-0.0426
•					1	1	0.5531	0. 226	0.379	-0.0823
0		!					0.5770	0. 205	0.422	-0.120
				/	/		0.6010	0. 185	0.465	-0.158
_	L					•	0.6249	0.165	0.506	-0.194
-0.2	. 1				/	Nr. P		*5*	* œ	*
					/		~	si 0	805 B	e ii
•							0.3854	-0.000	-0.000	0.0934
;					/		0.4093	0.0102	0.0140	0.0129
Š	L					_	0.4333	0.0178	0.0246	-0.0626
						_	0.4572	0.0228	0.0320	-0.133
	1					**	0.4812	0.0255	0.0363	-0.20]
						8in 8	0.5052	0.0259	0.0376	-0.265
9.0	ŀ						0.5291	0.0244	0.0360	-0.325
							0.5531	0.0209	0.0314	-0.383
_	1						0.5770	0.0156	0.0238	-0.437
C		_	_	_	_	_	0.6010	0.0086	0.0134	-0.489
	<u>-</u> ;0	0.2	0.3	9.0	0.5	9.0	0.6249	-0.000	0.000	-0.538
			*	7						

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6.6.2 ONE EDGE FREE AND ONE EDGE RESTRAINED IN THE TANGENTIAL DIRECTION

We shall specify the free edge to be χ_1 and the restrained edge to be χ_2 . The constants of integration are

$$C_{5} = -\frac{\left[1 + (\chi_{1})^{2}\right]^{\frac{3}{2}}}{3}$$

$$C_{4} = \frac{c_{3}(\chi_{1})^{2}}{2} - \frac{\left[1 - 4(\chi_{1})^{2}\right]\left[1 + (\chi_{1})^{2}\right]^{\frac{3}{2}}}{15}$$

$$C_{5} = \frac{1}{(\chi_{2})^{2}} \left[-f_{3}(\chi_{2}) + \sqrt{1 + (\chi_{2})^{2}} f_{4}(\chi_{2}) + c_{4} \left\{f_{2}(\chi_{2}) - \sqrt{1 + (\chi_{2})^{2}} f_{5}(\chi_{2})\right\} + c_{5} \left\{f_{1}(\chi_{2}) - \sqrt{1 + (\chi_{2})^{2}} f_{4}(\chi_{2})\right\}\right]$$

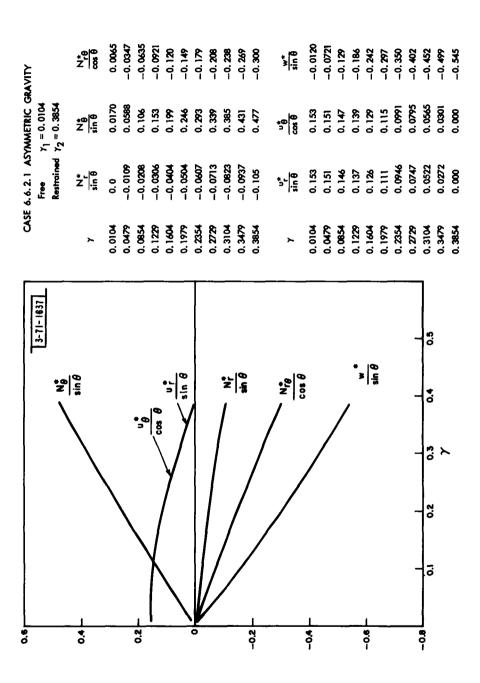
$$C_{6} = -\left\{f_{3}(\chi_{2}) + \frac{c_{5}(\chi_{2})^{2}}{2} + c_{4} f_{2}(\chi_{2}) + c_{5} f_{1}(\chi_{2})\right\}$$

$$6.6.2.4$$

The seven configurations which have been analyzed in this section are summarized in Table 6.6.2.1. The same discussion made in section 6.6.1 applies and no additional comments are required.

Table 6.6.2.1

Case	Free at 0,	Restrained at χ_2
6.6.2.1	.0104	. 3854
6.6.2.2	.0104	.6250
6.6.2.3	.0416	.62 5 0
6.6.2.4	.6250	. 3854
6.6.2.5	.0104	.9982
6.6.2.6	.1770	1.0243
6.6.2.7	.4010	1.0243



 CASE 6.6.2.2 ASYMMETRIC GRAVITY

 Free
 $\gamma_1 = 0.0104$

 Restrained
 $\gamma_2 = 0.625$

 N
 $\frac{1}{\sin \theta}$ $\frac{1}{\cos \theta}$

 0.0104
 -0.0082 0.0170 0.0065

 0.0173
 0.0890 -0.031 0.065

 0.0174
 -0.0064 0.242 -0.107

 0.1947
 -0.0645 0.242 -0.114

 0.3791
 -0.0645 0.342 -0.105

 0.3797
 -0.0645 0.346 -0.147

 0.3791
 -0.103 0.469 -0.147

 0.3791
 -0.104 0.346 -0.147

 0.5262
 -0.169 0.549 -0.145

 0.5239
 0.146 0.346 -0.346

 0.5249
 -0.195 0.765 -0.256

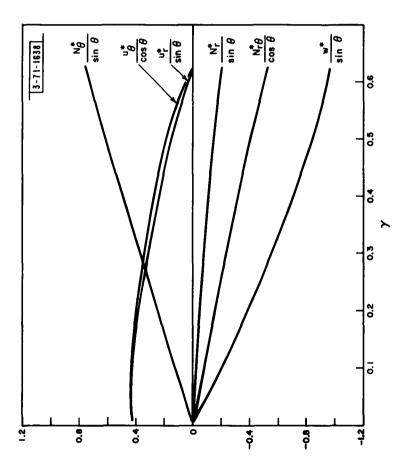
 0.5249
 -0.195 0.765 -0.256

 0.5249
 -0.195 0.765 -0.256

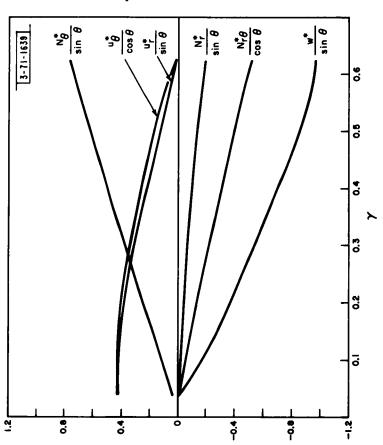
 0.5249
 -0.195 0.765 -0.256

 0.1034</th

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GRAVITY		*Z 8					-0.207				-0.405			sin 8	7									-0.912	
YMMETRIC	$y_1 = 0.0416$ $y_2 = 0.625$	X Sin 8	0.0416	0.117	0.192	0.266	0.339	0.411	0.483	0.554	0.624	0.694	0.764	*9 8	0.424	0.419	0.405	0.385	0.358	0.322	0. 278	0.225	0.161	0.0870	-0.000
CASE 6.6.2.3 ASYMMETRIC GRAVITY	Free Restrained	* 6	-0.000	-0.0171	-0.0348	-0.0518	-0.0689	-0.0867	-0.105	-0.125	-0.146	-0.169	-0.194	şi li	0.454	0.415	0.397	0.371	0.336	0. 295	0.247	0.193	0.133	0.0689	000
CASE		>	0.0416	0.0999	• 0.1583	0.2166	0. 2749	0, 3333	0.3916	0.4499	0.5083	0.5666	0.6249	>	0.0416	0.0999	0.1583	0.2166	0.2749	0.3333	0.3916	0.4499	0.5083	0.5666	0.6249



 CASE 6.6.2.4 ASYMMETRIC GRAVITY

 Free
 $\gamma_1 = 0.625$

 Restrained
 $\gamma_2 = 0.3854$
 $\gamma_1 = 0.625$ $N_{\rm P}^{\rm e}$
 $\gamma_1 = 0.625$ $N_{\rm P}^{\rm e}$

 0.6250
 -0.0029 0.625 -0.0004

 0.6010
 -0.0013 0.625 -0.0004

 0.5070
 -0.0029 0.625 -0.0004

 0.5371
 -0.0021 0.625 0.121

 0.5271
 -0.0027 0.540 0.121

 0.5271
 0.540 0.121 0.643

 0.4812
 -0.077 0.540 0.443

 0.4812
 -0.077 0.540 0.443

 0.4812
 -0.077 0.540 0.443

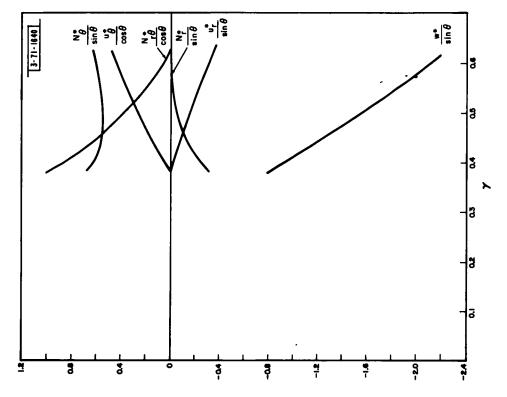
 0.4812
 -0.077 0.540 0.443

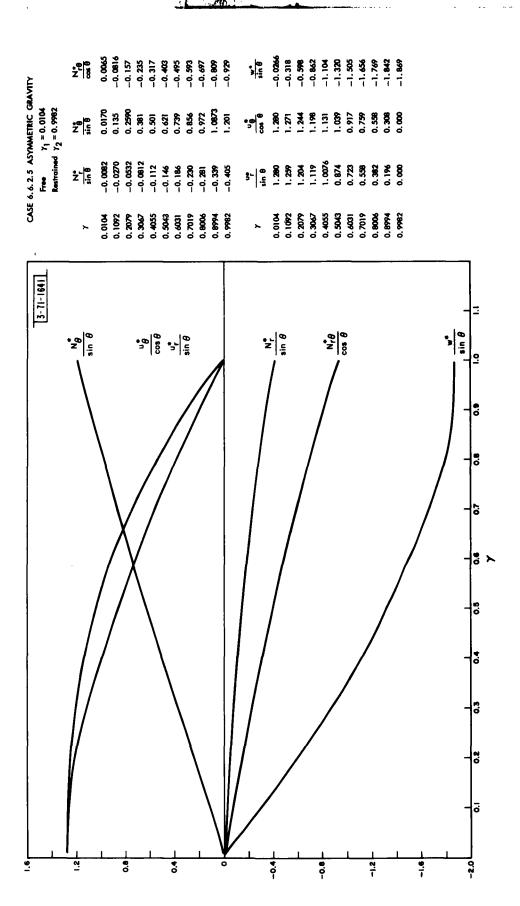
 0.4812
 -0.072 0.540 0.542

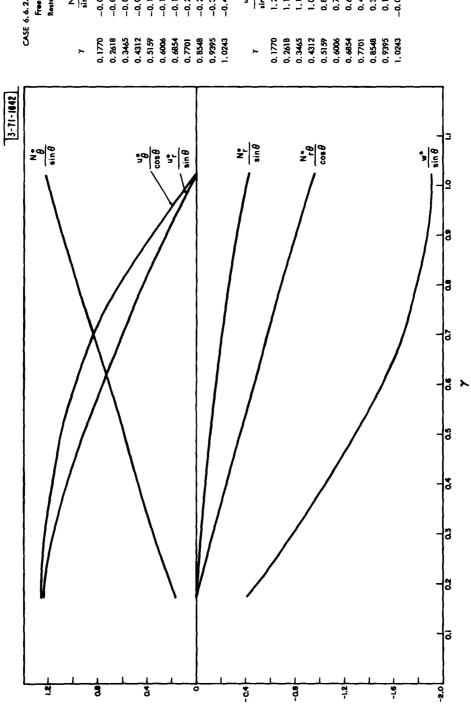
 0.4833
 -0.124 0.540 0.542

 0.4833
 -0.134 0.435 -1.946

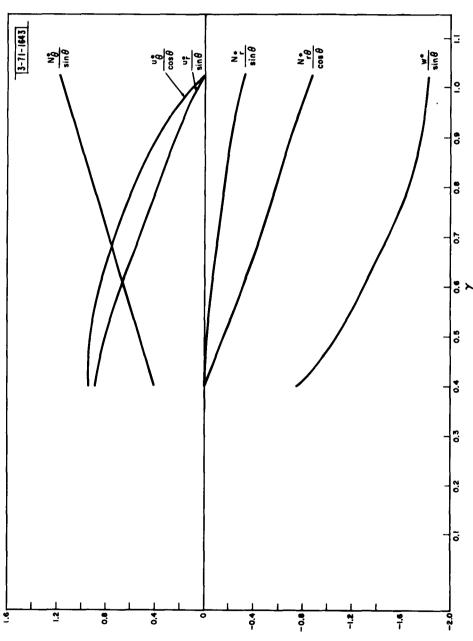
 0.5250
 -0.136 0.136







AVITY	.Φ Φ Ž 8	0.000	-0.118	-0.217	-0.305	-0.388	-0.468	-0.547	-0.626	-0.707	-0.789	-0.874	» is	-0.756	-0.942	-1.109	-1.28	-1.392	-1.509	-1.608	-1.690	-1.754	-1.38	-1.823
MAMETRIC GR $r_1 = 0.4010$ $r_2 = 1.0243$	χ *@ c.	0.401	0.470	0.547	0.628	0.708	0.789	0.868	0.947	1.0254	1.102	1.178	*0 8 80 8	0.937	0.926	0.897	0.853	0.791	0.712	0.614	0.496	0.355	0.1%	0.00
CASE 6. 6. 2.7 ASYMMETRIC GRAVITY Free $\gamma_1=0.4010$ Retroined $\gamma_2=1.0243$	ž .;	-0.000	-0.0087	-0.0284	-0.0539	-0.0831	-0.115	-0.150	-0.187	-0.22	-0.20	-0.316	,2- ië	0.880	0.825	0.761	0.688	0.607	0.520	0.426	0.327	0.223	0.114	0.000
CASE	>	0.4010	0.4633	0.5256	0.5880	0.6503	0.7126	0.7749	0.8373	0.8996	0.9619	1.0243	۰	0.4010	0.4633	0.5256	0.5880	0.6503	0.7126	0.7749	0.8373	0.8996	0.9619	1.0243



6.6.3 THE SHELL IS CLOSED AT THE APEX

The condition of finite stress resultants at the apex requires that

$$C_3 = \frac{-1}{3} \tag{6.6.3.1}$$

and

$$C_4 = \frac{-1}{15} \tag{6.6.3.2}$$

 ${\bf C}_5$ and ${\bf C}_6$ can again be obtained from equations (6.6.2.3) and (6.6.2.4).

The six configurations which have been analyzed in this section are summarized in Table 6.6.3.1, and the discussion of section 6.6.1 is applicable. No additional comments appear to be needed.

Table 6.6.3.1

Case	Restrained at χ_2
6.6.3.1	.2673
6.6.3.2	.3854
6.6.3.3	. 5 3 81
6.6.3.4	.6250
6.6.3.5	.803 8
6.6.3.6	1.0243

-0.07 -0.0271 -0.0464 -0.0657 -0.0852 -0.104 -0.124 -0.144 -0.184 CASE 6.6.3.1 ASYMMETRIC GRAVITY Closed at Apex $\gamma_1=0.0$ $r_2 = 0.2673$ 0.0450 0.0450 0.0772 0.108 0.141 0.173 0.205 0.205 0.237 0.332 0.0714 0.0689 0.0652 0.0630 0.0536 0.0457 0.0365 0.0368 0.0136 Restrained 0.0 -0.0089 -0.0154 -0.0219 -0.0285 -0.031 -0.0418 -0.0487 -0.0526 -0.0627 0.0713 0.0667 0.0647 0.0527 0.0446 0.0353 0.0248 0.0353 *⊃r | °ë 0.00 0.0361 0.0618 0.0874 0.1131 0.1388 0.1645 0.1902 0.1902 0.2416 0.0 0.0361 0.0618 0.0674 0.1131 0.138 0.1645 0.1902 0.1902 0.2416 3-71-1644 8 sin θ N LOS G S, is θ is θ χ nis θ nis

* 0 soo

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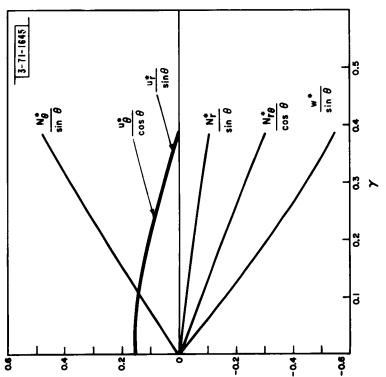
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RIC GRAVITY	$\gamma_1 = 0.0$ $\gamma_2 = 0.3854$	λ. Ν.	0.0	0598 -0.0359								4310.269	0.477 -0.300	υθ w* cos θ sin θ	-0.0			139 -0.187						0.0301 -0.500	
CASE 6.6.3.2 ASYMMETRIC GRAVITY	Closed at Apex $\gamma_1 =$ Restrained $\gamma_2 =$	χ is θ ris	0.0 0.0										-0.105 0.	v enis										0.0272 0.	
CASE		~	0.0	0.0479	0.0854	0.1229	0.1604	0.1979	0.2354	0.2729	0.3104	0.3479	0.3854	~	0.0	0.0479	0.0854	0.1229	0.1604	0.1979	0. 2354	0.2729	0.3104	0.3479	0 3854



CASE 6. 6. 3. 3 ASYMMETRIC GRAVITY

Closed of Apex $\gamma_1 = 0.0$ Restrained $\gamma_2 = 0.5381$ γ sin θ 1.0

0.0

0.0631

0.0

0.0631

0.067

0.144

0.199

0.2215

0.0429

0.2416

0.2743

0.0671

0.276

0.2743

0.0677

0.2743

0.0675

0.341

0.276

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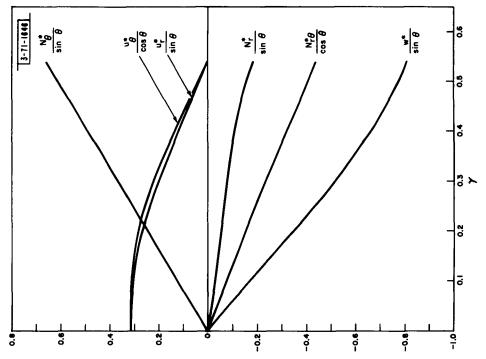
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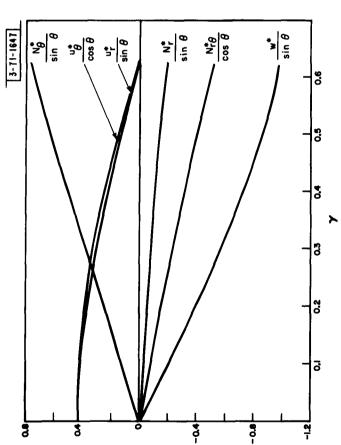
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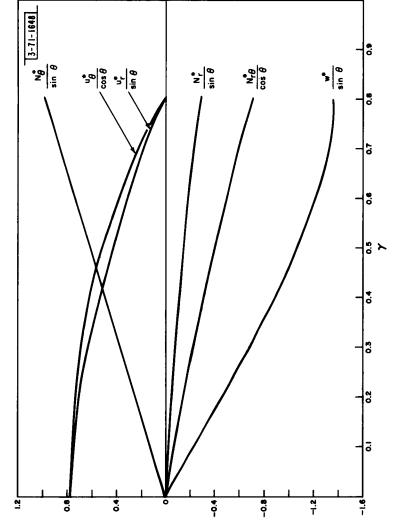
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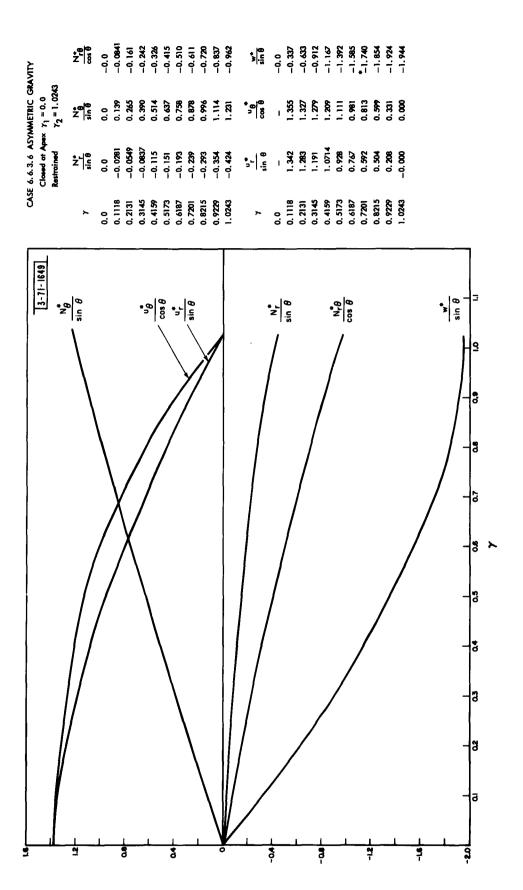




CASE 6.6.3.5 ASYMMETRIC GRAVITY Closed at Apax $\gamma_1 = 0.0$ Restrained $\gamma_2 = 0.8038$ γ $\frac{N_1^*}{\sin \theta}$ $\frac{N_1^*}{\sin \theta}$ $\frac{N_1^*}{\sin \theta}$ $\frac{N_1^*}{\sin \theta}$ 0.0 0.0 0.087 0.0225 0.112 0.248 0.0450 0.2045 0.248 0.0457 0.0450 0.057 0.057 0.057 0.057 0.057 0.057 0.057 0.057 0.057 0.059 0.071 0.059

Ac.





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